## Appendix K

Transportation Impact Study

# Imperial Avalon

Transportation Impact Study

Prepared for: Imperial Avalon, LLC

August 13, 2021

LB19-0002

FEHR PEERS

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### 1. Introduction

This report documents the assumptions, methodologies, and findings of a transportation impact study conducted by Fehr & Peers to evaluate the potential transportation impacts of the Imperial Avalon project (herein after referred to as the "Project") in the City of Carson, California, on a 27-acre site located to the west of Avalon Boulevard between the I-405 freeway interchange and 213<sup>th</sup> Street.

#### 1.1 Project Description

The Project is proposed to be developed in the City of Carson in the South Bay area of Los Angeles County on a site currently containing a mobile home park known as Imperial Avalon Mobile Estates. It is located approximately 17 miles south of downtown Los Angeles and approximately 6.5 miles east of the Pacific Ocean. The Project site is comprised of approximately 27 acres located to the west of Avalon Boulevard between the I-405 freeway interchange and 213<sup>th</sup> Street. The Project site is bounded by the District at South Bay development (currently under construction) and the Torrance Lateral Flood Control Channel to the north, the potential mixed-use Kott development to the east and south, and other residential uses to the west and south. Figure 1 illustrates the Project site plan.

The Project as analyzed in this study involves the construction of:

- 122 studio apartment units
- 368 one-bedroom apartment units
- 163 two-bedroom apartment units
- 380 townhome units
- 180 senior independent living units
- 10,352 square feet of restaurant space

The Project, as illustrated in the site plan in Figure 1, will have signalized access and egress at one main driveway location. This main Project driveway will provide access to and from Avalon Boulevard. Site access will also be served by two side-street stop-controlled driveways to and from Grace Avenue to the west. One additional stop-controlled right-turn-in/right-turn-out only driveway will be constructed along southbound Avalon Boulevard to the south of the main driveway.

#### 1.2 Study Scope

This transportation impact study will be incorporated into the environmental impact report (EIR) being prepared for the Project and follows the California Environmental Quality Act (CEQA) guidance for determining transportation impacts in accordance with Senate Bill (SB) 743. Since the City of Carson has not yet set its own vehicle miles traveled (VMT) metrics and thresholds, this study is consistent with the approach provided in the Governor's Office of Planning and Research (OPR) Technical Advisory on Evaluating



Transportation Impacts in CEQA (December 2018) and interim City guidance based on discussions with City staff.











Figure 1

### 2. SB 743 Overview

On September 27, 2013, Governor Jerry Brown signed SB 743 into law and started a process to fundamentally change transportation impact analysis conducted as part of CEQA compliance. OPR was charged with developing new guidelines for evaluating transportation impacts under CEQA using methods that no longer focus on measuring automobile delay and level of service (LOS). This change at the state level recognizes the unintended consequences of using LOS as an impact metric, which results in understating potential transportation impacts in greenfield areas and discouraging more sustainable infill projects and active transportation projects. SB 743 directs agencies to develop new guidelines that use a transportation performance metric which will help promote: the reduction of greenhouse gas emissions, the development of multimodal networks, and a more sustainable diversity of land uses.

OPR issued proposed updates to the CEQA guidelines in support of these goals in November 2017<sup>1</sup> and a supporting technical advisory in December 2018<sup>2</sup>. The updates establish VMT as the primary metric for evaluating a project's environmental impacts on the transportation system. The changes to CEQA guidelines Section 15064.3 to implement SB 743 were certified by the State in December of 2018. Lead agencies, including the City of Carson, have until July 2020 to implement these new requirements.

The City of Carson has not yet adopted new significance thresholds for transportation impacts based on VMT and has not yet revised its transportation impact assessment processes and guidelines accordingly. In lieu of City guidelines, VMT analysis based on the standard OPR guidance and interim City guidance based on discussions with City staff was conducted for the Project.

#### 2.1 VMT Analysis

The OPR technical advisory describes the four components of a VMT analysis necessary to comply with the new CEQA guidelines:

- VMT Screening & Qualitative Review: The first step is to determine when a VMT analysis is required. OPR recommends that projects be screened from a VMT analysis based on their size, location, and/or accessibility to transit.
- 2. VMT Analysis Methodology: If a project is not screened from requiring a VMT analysis, the City can use the regional travel demand model to estimate a project's VMT. OPR recommends that VMT be reported as "Home-Based VMT" per capita for residential projects and "Home-Based Work VMT" per employee for the employees of a project site.

<sup>&</sup>lt;sup>2</sup> State of California, Governor's Office of Planning and Research, *Technical Advisory on Evaluating Transportation Impacts in CEQA*, December 2018.



<sup>&</sup>lt;sup>1</sup> State of California, Governor's Office of Planning and Research, *Proposed Updates to the CEQA Guidelines, Final,* November 2017.

Home-Based VMT includes all vehicle roundtrips originating from the residence of the trip-maker. Home-Based Work VMT includes only vehicle roundtrips between the residence of the trip-maker and their place of work.

- 3. VMT Impact Thresholds: The City has discretion to develop and adopt its own VMT thresholds, or rely on thresholds recommended by other agencies, provided the decision of the lead agency to adopt such thresholds is supported by substantial evidence. OPR recommends that projects with VMT exceeding 15 percent below existing VMT per capita or per employee when compared to a regional or citywide average of these metrics may indicate project impacts. Fehr & Peers recommends utilizing citywide average VMT to determine the thresholds for this analysis, consistent with recent VMT analyses conducted for nearby developments and the City's General Plan Update.
- 4. **VMT Mitigation:** The types of mitigation that affect VMT are those that reduce the number of single-occupant vehicles generated by a project. Mitigation can be accomplished by altering the proposed land uses or by implementing transportation demand management (TDM) measures.

Table 1 presents a summary of the new SB 743 criteria and OPR's recommended guidance for VMT screening, analysis and thresholds. Since the City has yet to establish its own VMT significant impact thresholds and guidelines, the VMT analysis in this study conforms to the methodology shown in Table 1 and the OPR Technical Advisory.



### TABLE 1 SUMMARY OF CEQA GUIDANCE AND OPR TECHNICAL ADVISORY

CEQA Criteria	OPR Technical Advisory
VMT Screening	& Qualitative Review
If existing models or methods are not available to estimate VMT for the project being	Generally, qualitative analyses should only be conducted when methods do not exist for
considered, a lead agency may analyze the project's VMT qualitatively. Such a qualitative	undertaking a quantitative analysis. OPR suggests screening for small projects, retail uses
analysis would evaluate factors such as the availability of transit, proximity to other	less than 50 KSF, and projects located in high quality transit areas.
destinations, etc.	
VMT Analy	rsis Methodology
A lead agency has discretion to choose the most appropriate methodology to evaluate	OPR recommends reporting VMT as follows:
a project's VMT, including whether to express the change in absolute terms, per capita,	
per household or in any other measure.	Residential = Home-Based VMT per capita
	Office = Home-Based Work VMT per employee
A lead agency may use a model to estimate a project's VMT and may revise those	Retail = Change in total VMT
estimates to reflect professional judgment based on substantial evidence.	
	OPR also recommends using a regional travel demand model to estimate VMT.
VMT Imp	act Thresholds
Lead agencies have discretion to develop and adopt their own thresholds, or rely on	OPR recommends the following:
thresholds recommended by other agencies, provided the decision of the lead agency to	
adopt such thresholds is supported by substantial evidence.	Residential: A proposed project exceeding a level of 15% below existing regional or
	Citywide VMT per capita may indicate a significant transportation impact.
	Office: A proposed project exceeding a level of 15% below existing regional VMT per
	employee may indicate a significant transportation impact.
	Retail: A net increase in total VMT may indicate a significant transportation impact.

## 3. VMT Screening

VMT is heavily dependent on the land uses and location of a project. For example, a development site located in an urban area will typically have lower VMT because people have more options to walk, bike, take transit or drive short distances to nearby destinations in comparison to a suburban or rural environment where most people drive longer distances for their everyday work and household needs. Therefore, OPR has provided guidance related to several opportunities for screening projects that would generate low VMT as described in this chapter.

#### 3.1 Project Type Screening

Projects that generate less than 110 daily trips may be screened from conducting a VMT analysis. Local serving commercial uses less than 50,000 square feet may also be presumed to have a less than significant VMT impact absent substantial evidence to the contrary. This is because local serving commercial generally improves the convenience of shopping and dining close to home and has the effect of reducing vehicle travel. All the Project's commercial uses are less than 50,000 square feet. Therefore, the commercial component of the Project is identified as local serving and screened from VMT analysis.

#### 3.2 Low VMT Area Screening

Residential and office projects located within a low VMT generating area may be presumed to have a less than significant impact absent substantial evidence to the contrary. In addition, other employment-related and mixed-use land use projects may qualify for screening if the project can reasonably be expected to generate VMT per resident or per worker that is similar to the existing land uses in the low VMT area.

The Southern California Association of Governments (SCAG) Regional Travel Demand Model, which includes Los Angeles County and the City of Carson, is the most appropriate model to use for VMT forecasting within the City of Carson. This analysis used the SCAG model to measure the VMT performance for the Project's traffic analysis zone (TAZ) during Base Year 2016 (the most recently adopted SCAG base year<sup>3</sup>) conditions. TAZs are geographic polygons similar to Census block groups used to represent areas of homogenous travel behavior. The VMT metrics for the Project's TAZ are discussed in further detail below as part of the screening for residential land uses.

Low VMT areas for residential projects are defined as TAZs that generate VMT on a per capita basis that is at least 15% lower than the citywide average. Low VMT areas for office projects are defined as TAZs that generate VMT on a per employee basis that is at least 15% lower than the citywide average. The Project's TAZ is estimated to generate VMT per capita greater than 15% below the City's baseline VMT. Therefore,

<sup>&</sup>lt;sup>3</sup> Although an updated SoCal Connect RTP/SCS document was adopted by SCAG in 2020, they have not yet released the corresponding Base Year 2020 travel demand model data. Therefore, Base Year 2016 is still the latest existing conditions dataset.



the Project is not in an area with low residential VMT, which means the residential component of the Project cannot be presumed to have a less than significant VMT impact and may require further VMT analysis.

#### 3.3 Transit Priority Area (TPA) Screening

Projects located within one-half mile of either an existing major transit stop or a stop along an existing high quality transit corridor (HQTC) may also be exempt from VMT analysis<sup>4</sup>. Major transit stops are defined in the technical advisory as rail or bus rapid transit stations, ferry terminals served by transit, or the intersection of two HQTCs (defined as corridors with fixed-route bus service with no longer than 15-minute headways during peak commute periods).

Based on OPR guidance, projects located within a TPA may be presumed to have a less than significant impact absent substantial evidence to the contrary. However, this presumption may not be appropriate if the project:

- Has a Floor Area Ration (FAR) of less than 0.75
- Includes more parking for use by residents, customers, or employees than required by the City (unless additional parking is being provided for design feasibility, such as completing the floor of a subterranean or structured parking facility, or if additional parking is located within the project site to serve adjacent uses)
- Is inconsistent with the applicable SCS (as determined by the City)
- Replaces affordable residential units with a smaller number of moderate- or high-income residential units

The closest major transit stops to the Project are along the LA Metro Silver Line bus rapid transit route. However, the Project is more than one mile away from the closest Silver Line stop at the I-110/Carson Street interchange. Also, there are no HQTCs near the Project. Therefore, the Project is not within a transit priority area.

### 3.4 Screening Summary

Based on the screening criteria recommended by OPR, only the commercial component of the Project is exempt from VMT analysis. The residential component of the Project will need to be analyzed for potential VMT impacts.

<sup>&</sup>lt;sup>4</sup> California Code of Regulations, Title 14, Division 6, Chapter 3, Section 15064.3(b)(1)



## 4. VMT Analysis Methodology

For projects that do not meet any of the screening criteria described in the previous chapter, a VMT analysis is required. The VMT analysis relies on the best available data to inform trip generation and trip length estimates for each project land use. For projects consisting of residential, office, and commerical land uses, the VMT analysis can be conducted using the SCAG model. The SCAG 2016 RTP/SCS model was used to collect data and perform the VMT analysis for this study.

The following steps were undertaken to estimate the VMT generated by the Project.

#### 4.1 Step 1 – Determine Average Person Trip Rates

The SCAG model was used to estimate average person trip rates for the residential component of the Project. Person trip generation rates for each TAZ in the City were averaged to obtain a home-based production trip rate per resident. The home-based production person trip rate per resident is 1.7 for the City using this method. This study assumes these citywide average trip rates reasonably reflect the Project's person trip generation potential.

#### 4.2 Step 2 - Average Person Trip Rate to Vehicle Trips Conversion

Before conducting the VMT calculations, the person trips calculated in Step 1 need to be converted to vehicle trips. Average mode splits for the City were obtained from the SCAG model and used to estimate the Project's vehicle trip generation.

This step requires an estimate of the total resident and employee population of the Project. This study assumes the following average dwelling unit populations:

Studio Apartments
 One-Bedroom Apartments
 Two-Bedroom Apartments
 All Townhome Units
 All Senior Dwelling Units
 1.6 persons per household<sup>5</sup>
 2.0 persons per household<sup>6</sup>
 2.9 persons per household<sup>7</sup>
 3.6 persons per household<sup>8</sup>
 1.5 persons per household<sup>9</sup>

<sup>&</sup>lt;sup>9</sup> Based on City of Los Angeles VMT Calculator Tool blended senior/special needs rate



<sup>&</sup>lt;sup>5</sup> Based on U.S. Census Bureau American Community Survey (ACS) data for multifamily rental studio units in Los Angeles County (higher than the City of Carson rate – 1.3)

<sup>&</sup>lt;sup>6</sup> Based on U.S. Census Bureau ACS data for multifamily rental one-bedroom units in City of Carson

<sup>&</sup>lt;sup>7</sup> Based on U.S. Census Bureau ACS data for multifamily rental two-bedroom units in City of Carson

<sup>&</sup>lt;sup>8</sup> Based on Carson Housing Element, SCAG and CFD general/blended rate

These averages incorporate data from U.S. Census block groups in the surrounding area, the SCAG model and input from the Project development team and the City. Using these average population rates, the total resident population of the Project is 3,042.

Based on data from the SCAG model, this study estimates 92% of residential person trips occur in vehicles (not including bicycles or transit vehicles), with 47% trips in vehicles occupied by one person and the other 45% in vehicles occupied by an average of 2.5 people. The remaining 8% of residential person trips are forecasted to use other modes such as walking, biking, or transit, and as a result are not included in the VMT calculations. Using these estimates, the Project will generate 3,368 daily residential vehicle trips<sup>10</sup>.

#### 4.3 Step 3 - Estimate Trip Length

The trip lengths for the Project were estimated using data from the SCAG model. The SCAG model can produce average trip lengths for each TAZ in the City. For the TAZ including the Project site, the average trip length for home-based production trips is 9.1 miles.

#### 4.4 Step 4 - VMT Calculations

The final step to calculate VMT is to multiply the number of vehicle trips by the average trip length for those trips. The total VMT for the Project's residential uses is projected to be 30,649. The residential VMT is then divided by the 3,042 residents to obtain a VMT per capita of 10.1. These results are presented in Table 2 below.

<sup>&</sup>lt;sup>10</sup> The daily residential vehicle trips utilized in the VMT analysis is lower than the daily residential vehicle trips utilized for the Project's other CEQA impact analyses, such as air quality, greenhouse gas emissions, energy and noise. The other CEQA impact analyses utilized the ITE-based daily trip estimates as shown in the Appendix of this report. The ITE-based daily trip estimates are intended to be conservative to represent a worst-case scenario for assessing potential impacts to air quality, greenhouse gas emissions, energy and noise. The VMT analysis, which requires running the 2016 RTP/SCS SCAG model, must use the same methodology for deriving Project trip generation as was used to calculate the VMT threshold of significance, which in this case is the citywide average home-based VMT per capita from the 2016 RTP/SCS SCAG model. Therefore, to be internally consistent, the VMT analysis for this Project uses the SCAG model derived trip generation.



# TABLE 2 PROJECT VMT RESULTS HOME-BASED VMT PER CAPITA

Land Use	Population Trip Length Daily VMT (mi) Vehicle Trips		VMT	Home-Based VMT per Capita	
Residential	3,042	9.1	3,368	30,649	10.1

## 5. VMT Impact Thresholds

The 2016 RTP/SCS SCAG model was used to determine an appropriate baseline of VMT for projects in the City of Carson. The City's baseline VMT for Home-Based trips (per capita) is shown in Table 3.

Following the standard OPR guidance, a threshold of 15% below baseline VMT is used to determine if the Project will cause significant transportation impacts. If the Project generates VMT higher than this threshold, then it is expected to have a significant impact. If the Project generates VMT lower than this threshold, then it is expected to not have a significant impact.

Table 3 compares the City's baseline VMT with the residential VMT for the Project. The Home-Based VMT per capita for the Project (10.1) is 30% below the citywide average (14.4).

Based on the standard OPR thresholds and interim City guidance, the Project does not have any significant VMT impacts. The Project is estimated to generate VMT per capita of less than 15% below the citywide average for this metric. All commercial uses included in the Project are each less than 50,000 square feet and therefore identified as local serving. Since the Project has no significant impacts, no VMT mitigation is required.



## TABLE 3 PROJECT VMT IMPACT ASSESSMENT HOME-BASED VMT PER CAPITA

VMT Metrics	2016 Citywide Average [1]	Project Average [1]	% Below Citywide Average	Significant Impact?	
Home-Based VMT per Capita	14.40	10.10	30%	NO	

<sup>[1]</sup> Citywide and project VMT averages generated using the 2016 RTP/SCS SCAG travel demand model

### 6. Non-VMT Transportation Impacts

CEQA guidelines include several transportation impact categories in addition to the SB 743/VMT impact category discussed in the previous chapters. This chapter summarizes the Project's potential non-VMT transportation impacts.

#### **6.1 Freeway Safety Analysis**

Based on the *Interim LDIGR Safety Review Practitioners Guide* (December 2020), Caltrans requires an assessment of potential safety impacts to Caltrans facilities caused by the addition of project vehicle trips. The Caltrans NOP comment letter (included in the Appendix of this report) for the Project identified the following locations to be assessed for potential safety impacts:

#### • I-405/Avalon Blvd Interchange

- I-405 NB off-ramp to Avalon Blvd
- I-405 SB off-ramp to Avalon Blvd<sup>11</sup>
- NBL turn pocket from Avalon Blvd to I-405 NB on-ramp
- NB Avalon Blvd approach to I-405 SB on-ramp

#### • I-110/220<sup>th</sup> Street Interchange

- o I-110 NB off-ramp to 220<sup>th</sup> Street
- NBL turn pocket from Figueroa Street to I-110 NB on-ramp

For the off-ramp locations, a potentially significant safety impact is identified if the addition of project vehicle trips would result in an off-ramp queue that extends onto the freeway mainline. An off-ramp queue which extends onto the freeway mainline causes a potential safety concern if a significant speed differential exists between the off-ramp queue vehicles and the freeway mainline vehicles.

For the on-ramp locations, Caltrans has not identified a set of criteria for evaluating potential significant safety impacts. In lieu of such guidance, this memorandum summarizes collision data at these locations from the previous five years and turn pocket queue lengths with the addition of project vehicle trips. Connections, if any, between the collision data, turn pocket queue lengths and the addition of project vehicle trips causing potential safety concerns will then be identified.

<sup>11</sup> The I-405 Southbound Off-Ramp & Avalon Boulevard interchange will be reconfigured in the future to accommodate the District at South Bay project. This analysis considers the current traffic signal configuration at the I-405 Southbound Off-Ramp & Avalon Boulevard intersection for the Existing and Existing plus Project scenarios and the future traffic signal configuration at the I-405 Southbound Off-Ramp & New Internal District at South Bay Road (tentatively called Lenardo Drive) intersection for the Future (Year 2027) Base and Future (Year 2027) plus Project scenarios.



The off-ramp and on-ramp safety assessment is conducted for four traffic volume scenarios: Existing, Existing plus Project, Future (Year 2027) Base and Future (Year 2027) plus Project scenarios. The two future scenarios consider additional traffic volume from ambient growth (0.5% linear growth per year) and related projects in the City of Carson and unincorporated Los Angeles County. Trip generation and distribution for the Project and the related projects included in this analysis are shown in the Appendix of this report.

#### **6.1.1 Off-Ramp Locations**

To evaluate the adequacy of the existing and future off-ramp storage lengths, Caltrans guidance indicates that the 95<sup>th</sup> percentile queue length should be compared to the off-ramp storage length measured from the stop bar back to the gore point. This analysis utilizes the *Highway Capacity Manual (HCM)*, 6<sup>th</sup> Edition methodology to calculate the 95<sup>th</sup> percentile queue lengths.

Three freeway off-ramps were evaluated to determine whether the Project would create potentially significant freeway safety impacts. The three freeway off-ramps evaluated include:

- I-405 NB off-ramp to Avalon Blvd
- I-405 SB off-ramp to Avalon Blvd
- I-110 NB off-ramp to 220<sup>th</sup> Street

Queue lengths were estimated using the Synchro traffic analysis software package. Intersection counts were collected at the ramp locations and signal timing information from Caltrans was used to accurately analyze operations. Detailed results from this analysis can be found in the Appendix of this report.

Table 4 presents a summary of the off-ramp queuing analysis for Existing, Existing plus Project, Future (Year 2027) Base and Future (Year 2027) plus Project scenarios. As shown in Table 4, the freeway off-ramp queues do not exceed the storage length in any scenario or time period. Therefore, no significant freeway safety impact is identified at the off-ramp locations with the addition of Project trips.

#### **6.1.2 On-Ramp Locations**

As with the off-ramp locations, this analysis utilizes the *HCM*, 6<sup>th</sup> Edition methodology to calculate the 95<sup>th</sup> percentile queue lengths for the two left-turn pockets leading to freeway on-ramps identified in the Caltrans NOP comment letter for potential safety concerns. The two left-turn pockets evaluated include:

- NBL turn pocket from Avalon Blvd to I-405 NB on-ramp
- NBL turn pocket from Figueroa Street to I-110 NB on-ramp

Queue lengths were estimated using the Synchro traffic analysis software package. Intersection counts were collected at the on-ramp locations and signal timing information from Caltrans was used to accurately analyze operations. Detailed results from this analysis can be found in the Appendix of this report.

Table 5 presents a summary of the left-turn pocket queueing analysis for Existing, Existing plus Project, Future (Year 2027) Base and Future (Year 2027) plus Project scenarios. As shown in Table 5, the NBL turn pocket from Avalon Blvd to I-405 NB on-ramp has a queue exceeding the storage length in both Future



Base and Future plus Project scenarios. The NBL turn pocket from Figueroa Street to I-110 NB on-ramp has a queue exceeding the storage length in all four scenarios. While both left-turn pockets experience queuing issues, these findings suggest the queueing issues would occur even without the addition of Project trips. The addition of Project trips increases the forecast queue by approximately 1 car length on Avalon Boulevard. The Project trips are not expected to affect the Figueroa Street queue length.

To further assess potential safety concerns at the on-ramp locations, collision data was compiled for the previous five years utilizing the California Highway Patrol's Statewide Integrated Traffic Records System (SWITRS). The on-ramp locations where data was compiled include:

- NBL turn pocket from Avalon Blvd to I-405 NB on-ramp
- NB Avalon Blvd approach to I-405 SB on-ramp
- NBL turn pocket from Figueroa Street to I-110 NB on-ramp

Detailed summary tables and maps of collisions occurring near these locations are included in the Appendix of this report. As shown in these tables and maps, none of the collisions from the previous five years occur at or preceding the two left-turn pockets or the NB Avalon Blvd approach to I-405 SB on-ramp. There is a cluster of collisions which occurred on the I-405 NB on-ramp from Avalon Blvd, however based on the location of the collisions shown in the Appendix of this report these appear to be related to the SBR channelized merger rather than the NBL turn pocket. Therefore, no significant freeway safety impact is identified at the on-ramp locations with the addition of Project trips.

#### 6.2 Other CEQA Transportation Impact Categories

CEQA guidelines include several potential transportation impact categories other than VMT and freeway safety analysis, as documented in Appendix G, Section XVII of the CEQA Guidelines Appendices<sup>12</sup>. The remaining sections of this chapter summarize the other transportation impact categories and assess the Project for significant impacts under these categories.

#### 6.2.1 Programs, Plans, Ordinances and Policies

CEQA Guideline: "Would the project...Conflict with a program, plan, ordinance or policy addressing the circulation system, including transit, roadway, bicycle and pedestrian facilities?"

The Project does not prevent the addition of planned improvements to the City's circulation system as described in City documents including the City of Carson General Plan and Master Plan of Bikeways. The Project will not degrade facilities on the existing circulation system either. Therefore, the Project does not cause significant impacts for this category.

<sup>&</sup>lt;sup>12</sup> California Code of Regulations, Title 14, Division 6, Chapter 3, Sections 15000-15387



#### **6.2.2 Geometric Design Features and Incompatible Uses**

CEQA Guideline: "Would the project...Substantially increase hazards due to a geometric design feature (e.g. sharp curves or dangerous intersections) or incompatible uses (e.g. farm equipment)?"

The Project does not increase hazards due to a geometric design feature. All driveway access points are perpendicular to the public right-of-way and adequately spaced from existing signalized intersections. The construction of new intersections to serve the Project will conform to the latest *California Manual on Uniform Traffic Control Devices* (CAMUTCD) guidelines. The Project does not introduce incompatible uses with the surrounding community. Therefore, the Project does not cause significant impacts for this category.

#### **6.2.3 Emergency Access**

CEQA Guideline: "Would the project...Result in inadequate emergency access?"

The Los Angeles County Sheriff's Department provides law enforcement services to the City of Carson. The Carson branch of the Sheriff's Department is within a quarter mile of the Project site, to the south along Avalon Boulevard. The Los Angeles County Fire Department provides fire protection services to the City of Carson. The nearest fire station is within a 1.5-mile drive of the Project site, to the south on 223<sup>rd</sup> Street.

The Project provides several emergency access points from both Avalon Boulevard and Grace Avenue. The location and design of these access points, as well as the on-site internal roadways, would be designed to comply with applicable local requirements related to emergency vehicle access and circulation. Therefore, the Project does not cause significant impacts for this category.



TABLE 4
IMPERIAL AVALON PROJECT
FREEWAY OFF-RAMP QUEUE ANALYSIS

N/S Street Name	E/W Street Name	Ramp	Ramp Storage Analyzed		Existing	Existing Plu	us Project	Future Base	Future Plu	s Project
Ty 3 Street Name	z, w succervanie	Direction	Length (feet)	Period	95%ile queue length (feet) [1]	95%ile queue length (feet) [1]	Extended Queuing?	95%ile queue length (feet) [1]	95%ile queue length (feet) [1]	Extended Queuing?
Avalon Blvd	NB 405 Off-Ramp	NB	975	AM	50	50	NO	100	100	NO
A TV CHOTT BIVC	NB 103 On Ramp	113	373	PM	75	75	NO	150	150	NO
CD 405 Off Dames	Lamanda Du	SB	750	AM				150	175	NO
SB 405 Off-Ramp	Lenardo Dr	SB	750	PM				150	200	NO
Avalor Dive	CD 405 Off Dames	SB	1,000	AM	225	350	NO			
Avalon Blvd	SB 405 Off-Ramp	20	1,000	PM	175	350	NO			
Figures Ct	NR 110 Off Ramp	NB	1 150	AM	575	575	NO	700	700	NO
Figueroa St	NB 110 Off-Ramp	IND	1,150	PM	575	575	NO	850	850	NO

<sup>[1] 95</sup>th percentile queue lengths are rounded up to the next 25-foot increment based on the Synchro assumption of a 25 foot car length plus buffer space.

TABLE 5
IMPERIAL AVALON PROJECT
ON-RAMP LEFT-TURN POCKET QUEUE ANALYSIS

		L = {t /D; = l = t	Ramp		Existing	Existing Plu	us Project	Future Base	Future Plus	s Project
N/S Street Name	E/W Street Name	Left/Right Turn Pocket Movement	Storage Length (feet)	Analyzed Period	95%ile queue	95%ile queue length (feet) [1]	Extended Queuing?	95%ile queue length (feet) [1]	95%ile queue length (feet) [1]	Extended Queuing?
Avelen Dlvd	ND 405 Off Dames	NIDI	175	AM	150	175	NO	225	250	YES
Avalon Blvd	NB 405 Off-Ramp	NBL	175	PM	100	125	NO	200	225	YES
Figures Ct	NR 110 Off Ramp	NIDI	200	AM	850	850	YES	875	875	YES
Figueroa St	NB 110 Off-Ramp	INBL	NBL 300	PM	750	750	YES	775	775	YES

<sup>[1] 95</sup>th percentile queue lengths are rounded up to the next 25-foot increment based on the Synchro assumption of a 25 foot car length plus buffer space.

#### References

Highway Capacity Manual, 6th Edition, Transportation Research Board, 2016.

Enhancing Internal Trip Capture Estimation for Mixed-Use Developments, NCHRP Report 684

*Trip Generation, 10<sup>th</sup> Edition,* Institute of Transportation Engineers, 2017.

*Proposed Updates to the CEQA Guidelines, Final,* State of California, Governor's Office of Planning and Research, November 2017.

*Technical Advisory on Evaluating Transportation Impacts in CEQA,* State of California, Governor's Office of Planning and Research, December 2018.

California Code of Regulations, Title 14, Division 6, Chapter 3, Sections 15000-15387, State of California, 2020.

Interim LDIGR Safety Review Practitioners Guide, Caltrans, December 2020

City of Carson General Plan, City of Carson, October 2004

Carson Master Plan of Bikeways, City of Carson, August 2013



APPENDIX: Memorandum of Understanding



## Memorandum

Date: September 8, 2020

To: Ryan Kim, City of Carson

CC: Saied Naaseh and Gena Guisar, City of Carson

Darren Embry, Faring

From: Drew Heckathorn and Michael Kennedy, Fehr & Peers

Subject: Imperial Avalon CEQA Transportation Study Methodology and Assumptions

LB19-0002

This document summarizes the methodology and assumptions for the study to address potential transportation-related impacts and mitigation measures for the Imperial Avalon project's CEQA environmental documents.

#### **Overall Methodology**

OPR's *Technical Advisory on Evaluating Transportation Impacts in CEQA* will guide the transportation impact analysis in-lieu of City-adopted guidelines.

#### VMT Methodology

The SCAG 2016 RTP/SCS model will be utilized to collect data and perform the VMT analysis for this study. Specifically, the SCAG model analysis will estimate the following:

- Citywide average person trip rates per resident and per employee
- Citywide average travel mode split for resident and employee person trips
- Average trip lengths for home-based production trips and home-based work attraction trips for the TAZ containing the project site

This model-derived data will be utilized to estimate VMT per capita (home-based production trips) for the residents of the project and VMT per employee (home-based work attraction trips) for the employees of the project.



#### **Significant Impact Threshold**

Following the standard OPR guidance, a threshold of 15% below baseline VMT is used to determine if the project will cause significant transportation impacts. If the project generates VMT higher than this threshold, then it is expected to have a significant impact. If the project generates VMT lower than this threshold, then it is expected to not have a significant impact. The citywide average VMT per capita and VMT per employee will serve as the baseline VMT metrics for this analysis.

#### **Project Description**

The project as analyzed will include:

•	Multifar	mily Apartments	653 dwelling units
	0	Studios	122 dwelling units
	0	One-Bedroom	368 dwelling units
	0	Two-Bedrooms	163 dwelling units
•	Multifar	mily Townhomes	379 dwelling units
	0	Two-Bedrooms	192 dwelling units
	0	Three-Bedrooms	187 dwelling units
•	Indeper	ndent Living Senior Apartments	180 dwelling units
•	Restaur	ant Space	7,152 square feet

To estimate the project's resident population, the following rates were determined in consultation with City staff and the project development team (see attached RCLCO memorandum for details):

•	Studio Apartments	1.6 persons per household <sup>1</sup>
•	One-bedroom Apartments	2.0 persons per household <sup>2</sup>
•	Two-bedroom Apartments	2.9 persons per household <sup>3</sup>
•	All Townhouse Units	3.6 persons per household <sup>4</sup>
•	All Senior Dwelling Units	1.5 persons per household <sup>5</sup>

Therefore, the project is expected to serve a resident population of about 3,040.

The restaurant components of the project are screened out from VMT analysis since these are considered local serving (i.e. minimal VMT impact) based on OPR guidelines.

<sup>&</sup>lt;sup>1</sup> Based on U.S. Census Bureau American Community Survey (ACS) data for multifamily rental studio units in Los Angeles County (higher than the City of Carson rate – 1.3)

<sup>&</sup>lt;sup>2</sup> Based on U.S. Census Bureau ACS data for multifamily rental one-bedroom units in City of Carson

<sup>&</sup>lt;sup>3</sup> Based on U.S. Census Bureau ACS data for multifamily rental two-bedroom units in City of Carson

<sup>&</sup>lt;sup>4</sup> Based on Carson Housing Element, SCAG and CFD general/blended rate

<sup>&</sup>lt;sup>5</sup> Based on City of Los Angeles VMT Calculator Tool blended senior/special needs rate



#### **Other CEQA Transportation Study Components**

#### **Off-Ramp Queueing Analysis**

This analysis will assess the 95<sup>th</sup> percentile queue length at the following freeway off-ramps:

- 1. Avalon Blvd at I-405 northbound/southbound off-ramps
- 2. Carson Street at I-405 northbound/southbound off-ramps
- 3. 220th Street at I-110 northbound off-ramp
- 4. Carson Street at I-110 southbound off-ramp

The length of each off-ramp will be measured from the interchange intersection to the off-ramp gore point. Any queuing beyond 85% of this length will be considered a significant safety impact due to a potential speed differential between the mainline freeway traffic and the off-ramp traffic. Both direct and cumulative impacts will be assessed. HCM, 6<sup>th</sup> Edition methodology and Synchro software will be utilized for this analysis.

#### Other Appendix G Transportation Items

A qualitative assessment of the other transportation items in the CEQA Appendix G checklist will be conducted to determine any non-VMT significant impacts.

#### **Mitigation Measures**

If the project creates any transportation-related significant impacts, potential mitigation measures will be explored. For VMT impacts, mitigation can be accomplished by altering the proposed land uses or by implementing transportation demand management (TDM) measures. Potential TDM measures will be considered first. Altering the proposed land uses will only be considered if TDM measures are not sufficient to mitigate potential impacts.

# APPENDIX: RCLCO Memorandum



### **MEMORANDUM**

DATE: August 25, 2020

TO: CSG Consultants, Inc. – Gena Guisar, AICP, Principal Planner

FROM: RCLCO – Derek Wyatt, Managing Director

SUBJECT: Household Size Analysis for New Apartment Development in Carson, California.

CSG Consultants, Inc. ("CSG" and "Client") is providing contract planning services to the City of Carson, California. One of the proposed projects in CSG's Carson portfolio is Faring's proposed large-scale residential development located at 21207 South Avalon Boulevard in Carson. The project is planned to be comprised of 70% rental apartment units and 30% for-sale townhomes. As part of the planning process, the City has requested a concurrent study that estimates the expected occupancy / household sizes of the various unit types and sizes encompassed in the project's apartment development program (i.e., studio, one-bedroom, two-bedroom, etc.).

The city relies on household sizes determined by the Fiscal Impact Analysis, dated March 2019 and prepared by NBS Government Finance Group. The FIA distinguishes two types of residential development in the City of Carson:

- 1. Residential Projects Studio/Apartment Projects under this category consist of one-bedroom or less (studios) and assumes a person per unit household size of 2.0
- 2. Residential Projects All Other Projects under this category are defined as two-bedroom or more and assume a person per unit household size of 3.6.

It is important to note that the household size estimates for the residential project categories described above only distinguish by unit size (i.e., one-bedroom, two-bedroom, etc.) and do not categorize projects by other characteristics that have a meaningful impact on occupancy ratios, such as tenure (own vs. rent) or product type (single-family vs. multifamily).

To evaluate the appropriate occupancy metrics for the rental apartment units in the proposed project, CSG engaged RCLCO to analyze actual household sizes for comparable product in Carson and the broader Los Angeles County region.

According to demographic data from Esri, one of the premier providers of Geographic Information System (GIS) software and demographic data, Carson is estimated to have a population of 93,600 and 25,825 households in 2020, which equates to an average household size of 3.62 for the city overall. It is important to note that this occupancy characteristic is likely to vary for different household and unit types. For example, approximately three-quarters of the households in the city own versus rent, which skews the data relative to the proposed project, which is planned to be comprised of approximately 70% rental apartment units and 30% for-sale townhomes.

To explore the demographics of the market for the rental apartment units at a deeper level, RCLCO relied on the U.S. Census Bureau's American Community Survey ("ACS"). Data was collected through the Public Use Microdata Sample ("PUMS") files, which are a set of untabulated records about individual people or housing units. This data set allows for more granular tabulation of household sizes based on a variety of household characteristics.

<sup>1</sup> http://ci.carson.ca.us/content/files/pdfs/planning/docs/projects/CFD/NBSFiscalStudy032019.pdf

<sup>&</sup>lt;sup>2</sup> https://www.esri.com/en-us/arcgis/products/data/data-portfolio/demographics

<sup>&</sup>lt;sup>3</sup> https://www.census.gov/programs-surveys/acs

<sup>&</sup>lt;sup>4</sup> https://www.census.gov/programs-surveys/acs/data/pums.html



For the purposes of the proposed project, the RCLCO analysis focused on households that <u>rent in multifamily buildings</u> (defined as having five or more units). Considering this is a relatively limited sample set for the City of Carson, the analysis was also completed for all of Los Angeles County as a reference point. The average household sizes for the various unit types included in the proposed project's development program are shown in the table below:

#### Household Size by Unit Type for Renters in Multifamily Buildings (5+ Units)

UNIT TYPE	CITY OF CARSON	LOS ANGELES COUNTY
Studio	1.32	1.60
1BR	1.96	1.85
2BR	2.86	2.78
3BR	2.91	3.59

The metrics shown in the table above provide appropriate assumptions for estimating occupancy characteristics for the proposed project's rental apartment units considering the likely household composition by unit type, filtered for location (Carson and Los Angeles County), tenure (rent vs. own), and product type (multifamily vs. single-family).



#### **General Limiting Conditions**

Reasonable efforts have been made to ensure that the data contained in this study reflect accurate and timely information and are believed to be reliable. This study is based on estimates, assumptions, and other information developed by RCLCO from its independent research effort, general knowledge of the industry, and consultations with the client and its representatives. No responsibility is assumed for inaccuracies in reporting by the client, its agent, and representatives or in any other data source used in preparing or presenting this study. This report is based on information that to our knowledge was current as of the date of this report, and RCLCO has not undertaken any update of its research effort since such date.

Our report may contain prospective financial information, estimates, or opinions that represent our view of reasonable expectations at a particular time, but such information, estimates, or opinions are not offered as predictions or assurances that a particular level of income or profit will be achieved, that particular events will occur, or that a particular price will be offered or accepted. Actual results achieved during the period covered by our prospective financial analysis may vary from those described in our report, and the variations may be material. Therefore, no warranty or representation is made by RCLCO that any of the projected values or results contained in this study will be achieved.

Possession of this study does not carry with it the right of publication thereof or to use the name of "Robert Charles Lesser & Co." or "RCLCO" in any manner without first obtaining the prior written consent of RCLCO. No abstracting, excerpting, or summarization of this study may be made without first obtaining the prior written consent of RCLCO. This report is not to be used in conjunction with any public or private offering of securities or other similar purpose where it may be relied upon to any degree by any person other than the client without first obtaining the prior written consent of RCLCO. This study may not be used for any purpose other than that for which it is prepared or for which prior written consent has first been obtained from RCLCO.

APPENDIX: Caltrans NOP Comment Letter

#### DEPARTMENT OF TRANSPORTATION

DISTRICT 7 100 S. MAIN STREET, MS 16 LOS ANGELES, CA 90012 PHONE (213) 269-1124 FAX (213) 897-1337 TTY 711 www.dot.ca.gov



February 23, 2021

Ms. Gena Guisar, Contract Planner City of Carson, Planning Division Community Development Department 701 East Carson Street Carson, CA 90745

RE: Imperial Avalon Mixed-Use Project Vic. LA-405 PM 11.24, LA-110 PM 6.77 SCH # 2021010116 GTS # LA-2021-03470AL-NOP

Dear Ms. Guisar:

Thank you for including the California Department of Transportation (Caltrans) in the environmental review process for the above referenced project. The Project would involve removal of the existing on-site uses and construction of two residential apartment buildings and two residential apartment mixed use buildings within the eastern half of the Project site, and 48 townhome buildings within the western half of the Project site. The Project would allow for the construction of 833 residential units within the four apartment buildings, 180 of which would be age-restricted for senior residents. The apartment buildings would contain a mix of studio, one-bedroom, and two-bedroom units. The Project would also include construction of 380 dwelling units within the townhome portion of the Project. The townhomes would consist of a mix of two- and three-bedroom units. A total of 1,213 residential dwelling units would be provided. The residential apartment mixed use buildings would contain one restaurant and one café respectively. The project is immediate adjacent to the I-405/Avalon Blvd. interchange.

The mission of Caltrans is to provide a safe and reliable transportation network that serves all people and respects environment. Senate Bill 743 (2013) has been codified into CEQA law. It mandates that CEQA review of transportation impacts of proposed developments be modified by using Vehicle Miles Traveled (VMT) as the primary metric in identifying transportation impacts. As a reminder, Vehicle Miles Traveled (VMT) is the standard transportation analysis metric in CEQA for land use projects after the July 1, 2020 statewide implementation date. You may reference The Governor's Office of Planning and Research (OPR) website for more information.

http://opr.ca.gov/ceqa/updates/guidelines/

Ms. Gena Guisar, Contract Planner February 23, 2021 Page 2 of 4

This development should incorporate multi-modal and complete streets transportation elements that will actively promote alternatives to car use and better manage existing parking assets. Prioritizing and allocating space to efficient modes of travel such as bicycling and public transit can allow streets to transport more people in a fixed amount of right-of-way.

Caltrans supports the implementation of complete streets and pedestrian safety measures such as road diets and other traffic calming measures. Please note the Federal Highway Administration (FHWA) recognizes the road diet treatment as a proven safety countermeasure, and the cost of a road diet can be significantly reduced if implemented in tandem with routine street resurfacing.

Also, Caltrans has published the VMT-focused Transportation Impact Study Guide (TISG), dated, May 20, 2020 and Caltrans Interim Land Development and Intergovernmental Review (LD-IGR) Safety Review Practitioners Guidance, prepared on December 18, 2020.

https://dot.ca.gov/programs/transportation-planning/office-of-smart-mobility-climate- change/sb-743

Overall, the environmental report should include a Transportation Impact Study (TIS) to ensure all modes are well served by planning and development activities. This includes reducing single occupancy vehicle trips, ensuring safety, reducing vehicle miles traveled, supporting accessibility, and reducing greenhouse gas emissions.

We encourage the Lead Agency to evaluate the potential of Transportation Demand Management (TDM) strategies and Intelligent Transportation System (ITS) applications in order to better manage the transportation network, as well as transit service and bicycle or pedestrian connectivity improvements.

For additional TDM options, please refer to the Federal Highway Administration's Integrating Demand Management into the Transportation Planning Process: A Desk Reference (Chapter 8). This reference is available online at:

http://www.ops.fhwa.dot.gov/publications/fhwahop12035/fhwahop12035.pdf

The proposed 1,213 residential dwelling units may have a traffic safety impact at the following locations:

Ms. Gena Guisar, Contract Planner February 23, 2021 Page 3 of 4

- 1. I-405 NB off-ramp to Avalon Blvd and related intersection
- 2. I-405 NB on-ramp from NB Avalon Blvd and related intersection

These locations have been identified in Caltrans network screening process' as having existing traffic safety impacts and planned safety improvements. Additional analysis will be required when more detailed project trip data is provided to determine if additional safety countermeasures may be warranted for project identified safety impacts.

Freeway queuing analysis may be needed at the following ramp locations once more detailed project traffic data is available.

- 3. I-405 SB off-ramp to Avalon Blvd.
- 4. I-110 NB off-ramp to W 220<sup>th</sup> Street.

The project may conduct its own analysis to determine significant freeway queuing traffic safety impacts as outlined within the Caltrans Interim Land Development and Intergovernmental Review (LD-IGR) Safety Review Practitioners Guidance and provide that analysis as a submittal for review, or Caltrans will conduct such analysis once the required project trip data is provided.

5. Per a conversation between the City and Caltrans staffs on February 10, 2020, both agencies agree that a traffic safety analysis for the off-ramps and turn pockets should be conducted for the above study locations in addition to I-405 SB on-ramp from NB Avalon Blvd. (right-turn pocket).

If a potential safety impacts are identified, the following preferred traffic safety impact mitigation may be recommended as mitigation:

- Transportation demand management program(s) to reduce the traffic safety impacts, which may include increased transit access, commute trip reductions such as rideshare programs, shared mobility facilities (bicycle or vehicular), increased bicycle and pedestrian infrastructure;
- Investments to existing active transportation infrastructure, or transit system amenities (or expansion) to reduce the project's traffic safety impacts; and/or
- Potential change(s) to the ramp terminal operations including, but not limited to lane reassignment, traffic signalization, signal phasing or timing modifications, turn lane extensions to mitigate safety impacts from project traffic.

Caltrans recommends that project traffic data be provided as soon as it is available to allow for adequate time to review and provide analysis.

Ms. Gena Guisar, Contract Planner February 23, 2021 Page 4 of 4

Any proposed changes to any infrastructure within Caltrans right of way will require an encroachment permit. This work will require additional review and may be subject to additional requirements to ensure current design standards and access management elements are being addressed.

Caltrans recommends early coordination on safety analysis and encroachment permit, if needed, so findings can be considered in the Draft Environmental Impact Report. If you have any questions, please feel free to contact Mr. Alan Lin the project coordinator at (213) 269-1124 and refer to GTS # LA-2021-03470AL-NOP.

Sincerely,

MIYA EDMONSON

Miya Edmonson

IGR/CEQA Branch Chief email:

**State Clearinghouse** 

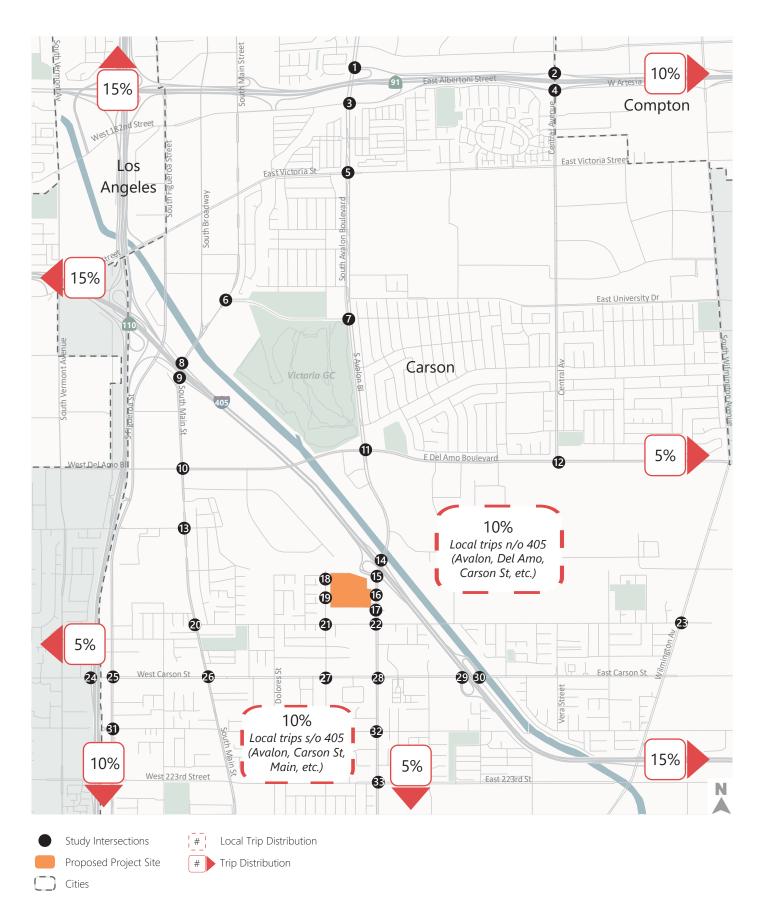
APPENDIX: Trip Generation

### IMPERIAL AVALON PROJECT ESTIMATED PROJECT TRIP GENERATION

	ITE Land					Generation Rates [a]				Estimated Trip Generation						
Land Use	Use Code	Size		Al.	И Peak Ho	ur	PI	И Peak Ho	our		AM	Peak Hour	Trips	PM	Peak Hour	Trips
	Use Code		Daily	Rate	In%	Out%	Rate	In%	Out%	Daily	In	Out	Total	In	Out	Total
PROPOSED PROJECT																
Multifamily Housing (Mid-Rise)	221	1,033 DU	5.44	0.36	26%	74%	0.44	61%	39%	5,620	97	275	372	278	177	455
Less: Internal capture			5%		5%	9%		4%	7%	(281)	(5)	(25)	(30)	(11)	(12)	(23)
Less: Walk/Bike/Transit Credit [b]			0%	0%			0%			0	0	0	0	0	0	0
Net External Vehicle Trips										<u>5,339</u>	<u>92</u>	<u>250</u>	<u>342</u>	<u>267</u>	<u>165</u>	432
Ovality Bastayrant	931	8.47 KSF	83.84	0.73	55%	45%	7.8	67%	220/	710	3	3	6	44	22	66
Quality Restaurant	951	0.47 NSF		0.73	20%	45% 4%	7.0	14%	33% 18%	_						
Less: Internal capture			11%	001	20%	4%	00/	14%	18%	(78)	(1)	0	(1)	(6)	(4)	(10)
Less: Walk/Bike/Transit Credit [b]			0%	0%			0%			0	0	0	0	0	0	0
Total Driveway Trips										<u>632</u>	<u>2</u>	<u>3</u>	<u>5</u>	<u>38</u>	<u>18</u>	<u>56</u>
Less: Pass-by			43%	43%			43%			(272)	(1)	(1)	(2)	(16)	(8)	(24)
Net External Vehicle Trips										<u>360</u>	<u>1</u>	<u>2</u>	<u>3</u>	<u>22</u>	<u>10</u>	<u>32</u>
Coffee/Donut Shop without Drive-Through Window [c]	936	1.882 KSF	346.23	101.14	51%	49%	36.31	50%	50%	652	97	93	190	34	34	68
Less: Internal capture	330	1.002 131	10%	101.14	20%	4%	30.31	14%	18%	(65)	(19)	(4)	(23)	(5)	(6)	(11)
				00/	20%	4%	00/	14%	10%							
Less: Walk/Bike/Transit Credit [b]			0%	0%			0%			0	0	0	0	0	0	0
Total Driveway Trips										<u>587</u>	<u>78</u>	<u>89</u>	<u>167</u>	<u>29</u>	<u>28</u>	<u>57</u>
Less: Pass-by			43%	43%			43%			(252)	(34)	(38)	(72)	(12)	(12)	(24)
Net External Vehicle Trips										<u>335</u>	<u>44</u>	<u>51</u>	<u>95</u>	<u>17</u>	<u>16</u>	<u>33</u>
Senior Adult Housing - Attached	252	180 DU	3.85	0.2	35%	65%	0.26	55%	45%	693	13	23	36	26	21	47
Less: Internal capture	202	.00 20	0%	0.2	0%	0%	0.20	0%	0%	0	0	0	0	0	0	0
Less: Walk/Bike/Transit Credit [b]			0%	0%	070	070	0%	070	070	0	0	0	0	0	0	0
Net External Vehicle Trips			070	076			076			693	13	<u>23</u>	<u>36</u>		<u>21</u>	47
Net External Venicle Trips										093	13	<u> 23</u>	<u>30</u>	<u>26</u>	<u> </u>	47
TOTAL DRIVEWAY TRIPS										7,251	<u>185</u>	<u>365</u>	<u>550</u>	<u>360</u>	232	<u>592</u>
TOTAL PROJECT EXTERNAL VEHICLE TRIPS										<u>6,727</u>	<u>150</u>	<u>326</u>	<u>476</u>	<u>332</u>	212	<u>544</u>
EXISTING USE CREDIT																
EXISTING USE CREDIT																
Mobile Home Park	-	225 DU	-	-	-	-	-	-	-	1,141	25	49	74	49	38	87
	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-
	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-
TOTAL EXISTING DRIVEWAY TRIPS [d]										<u>1,141</u>	<u>25</u>	<u>49</u>	<u>74</u>	<u>49</u>	<u>38</u>	<u>87</u>
NET INCREMENTAL EXTERNAL TRIPS										5,586	125	277	402	283	174	457

#### Notes

- [a] Source: Institute of Transportation Engineers (ITE), Trip Generation, 10th Edition, 2017 and NCHRP 8-51, Internal Trip Capture Estimation Tool, Texas Transportation Institute.
- [b] A 0% Walk/Bike/Transit Credit was used based on the site's general suburban context.
- [c] ITE use 933 Fast-Food Restaurant without Drive-Through Window used for daily rate due to lack of daily rate data for ITE use 936 Coffee/Donut Shop without Drive-Through Window.
- [d] 24-hour counts were taken at existing driveways at the proposed Project site in lieu of using estimated existing trips from ITE Trip Generation.





# IMPERIAL AVALON PROJECT RELATED PROJECTS

					Trip Generation						
No.	Project Location	Land Use	!	Size			AM			PM	
					Daily	IN	OUT	TOTAL	IN	OUT	TOTAL
1	CSUDH Master Plan	Mixed Use	[1]	[1]	N/A	2,299	1,415	3,714	1,940	2,286	4,226
2	The District at South Bay	Mixed Use	[1]	[1]	57,589	1,469	1,086	2,555	2,224	2,060	4,284
3	Jefferson at Avalon	Mixed Use	[1]	[1]	10,854	190	350	540	431	295	726
4	21138 S Western Ave	Gas Station	12	fp	1,461	56	48	104	52	51	103
5	Union South Bay (21521 S Avalon Blvd)	Multifamily	357	du	2,613	38	126	164	126	74	200
	Official South Bay (21321 3 Avaion Biva)	Shopping	32,000 ksf		1,208	19	11	30	59	63	122
6	University Village	Shopping	47,000	ksf	1,774	27	17	44	86	93	179
7	1007 E Victoria St	Multifamily	38	du	278	4	13	17	13	8	21
8	NEC Victoria and Central	Multifamily	175	du	1,281	19	62	81	62	36	98
9	2254 E 223rd St	Warehousing	120,500	ksf	210	16	5	21	6	17	23
10	21900 S Wilmington Ave	Warehousing	411,840		717	54	16	70	21	57	78
11	Veterans Village	Multifamily		du	373	5	18	23	18	11	29
	veteraris vinage	General Housing	15,000		146	15	2	17	3	14	17
12	21809-21811 S Figueroa St	Multifamily		du	234	3	11	14	11	7	18
13	140 W 223rd st	Multifamily	2	du	15	0	1	1	1	0	1
14	123 E 223rd st	Multifamily	9	du	66	1	3	4	3	2	5
15	22410 Vermont Ave	Multifamily		du	300	4	15	19	14	8	22
16	939 W 223rd St	Warehousing	5,820	ksf	10	1	0	1	0	1	1
17	345/369 E 220th St	Multifamily	35	du	256	4	12	16	12	7	19
18	1054 W 204th St	Public Park	9	Acre	7	0	0	0	1	0	1
19	20850 Normandie Ave	Warehousing	204	ksf	469	36	14	50	12	41	53
20	402 Sepulveda Blvd	Senior Adult Housing	65	du	278	5	10	15	12	8	20
21	Carol Kimmelman Campus	Mixed Use	[1]	[1]	3,808	105	83	188	244	192	436
22	Creek Dominguez Hills	Mixed Use	[1]	[1]	16,132	580	384	964	727	669	1,396
23	21801 Vera St	Single Family	18	du	170	3	10	13	11	7	18
24	Carson Arts Project	Multifamily	46	du	337	5	16	21	16	10	26
25	Harbour UCLA Medical Center	Mixed Use	[1]	[1]	1,620	166	34	200	33	164	197
				Total	102,206	5,124	3,762	8,886	6,138	6,181	12,319

#### Notes:

du = dwelling unit

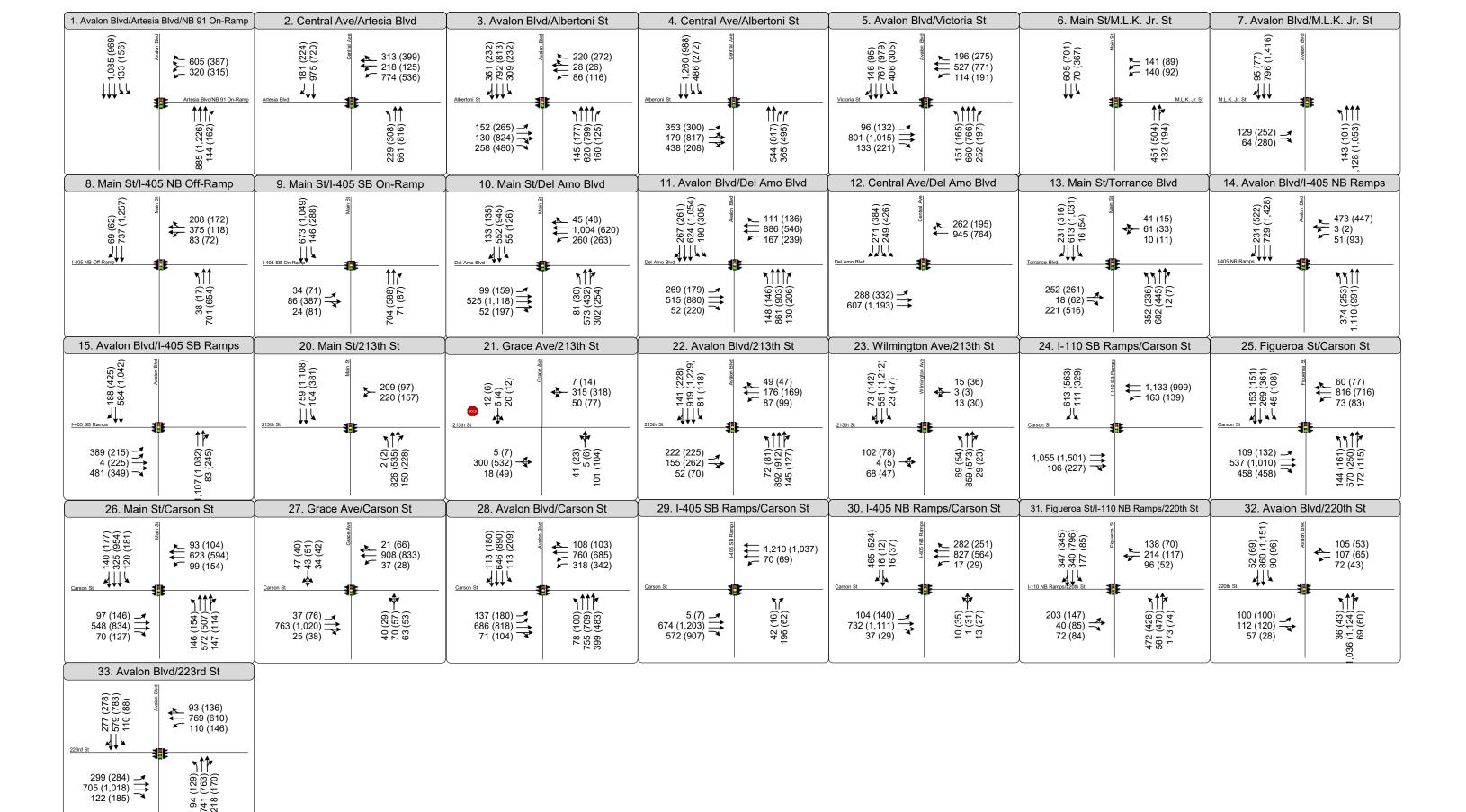
ksf = one thousand square feet

fp = fueling positions

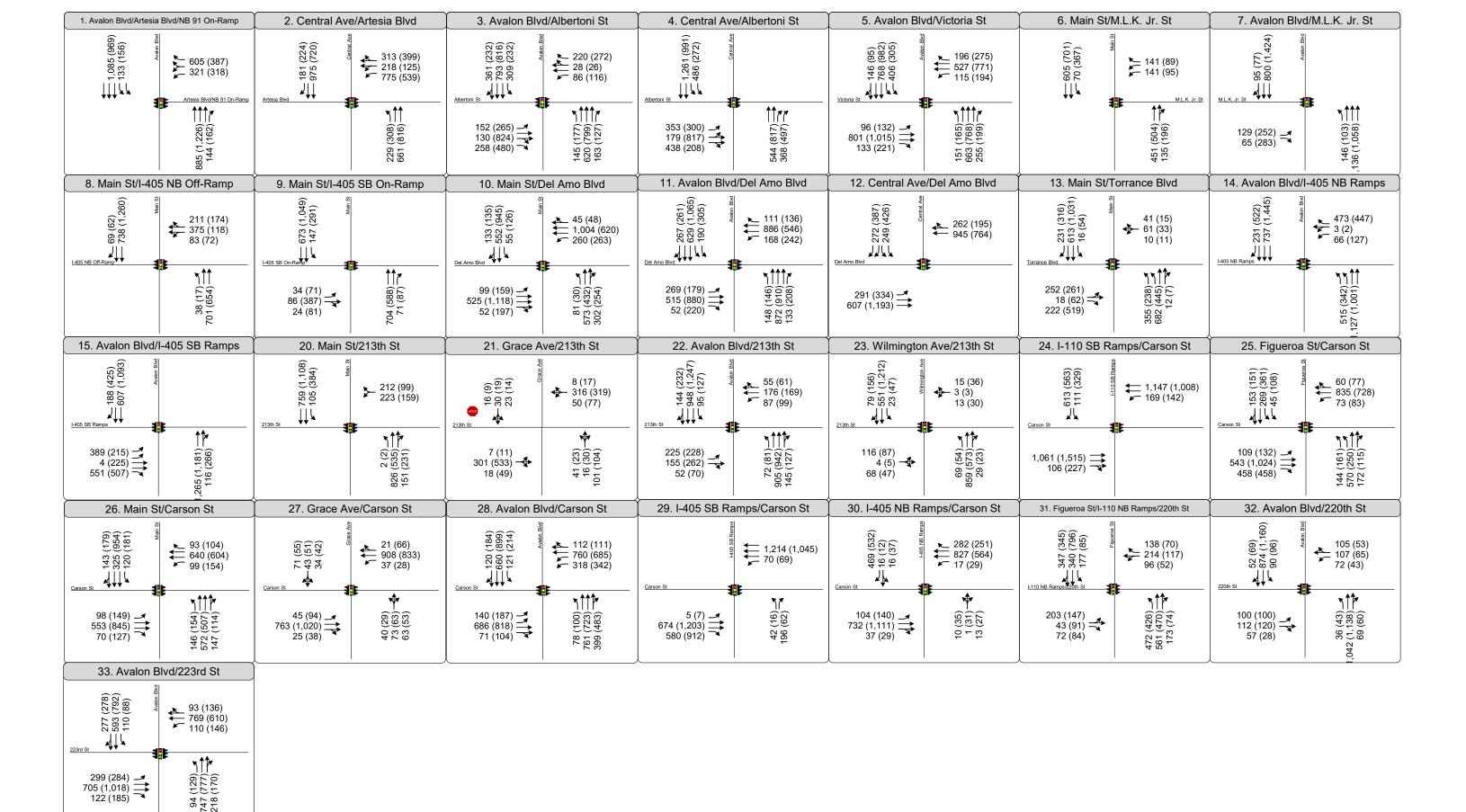
Related projects list is based on information provided by the City of Carson, the County of Los Angeles, publicly available environmental documentation, and trip generation rates contained in ITE Trip Generation, 10th Edition.

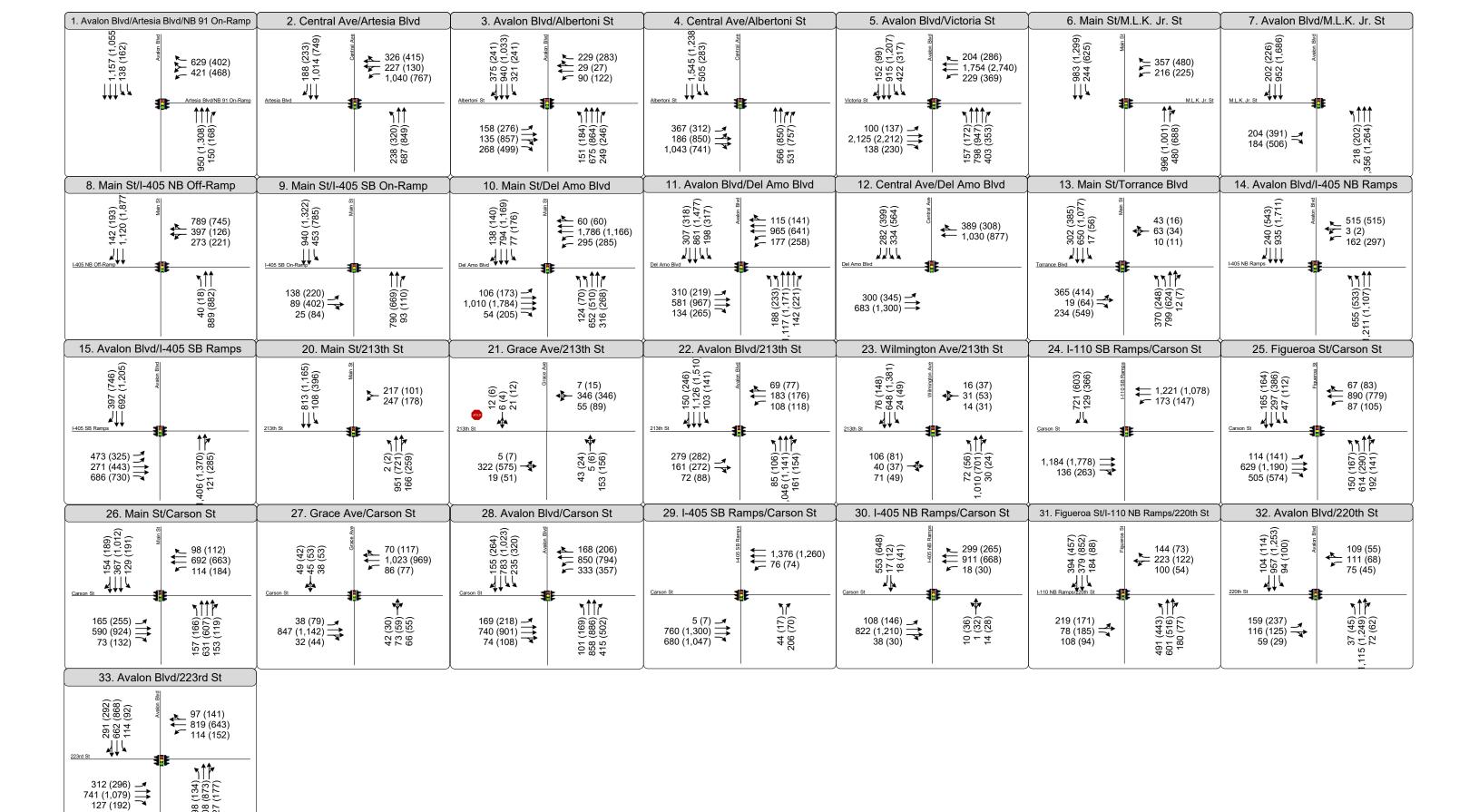
<sup>[1]</sup> Mixed Use developments contain more than one independent variable to calculate trip generation.

APPENDIX: Lane Configurations and Traffic Volumes



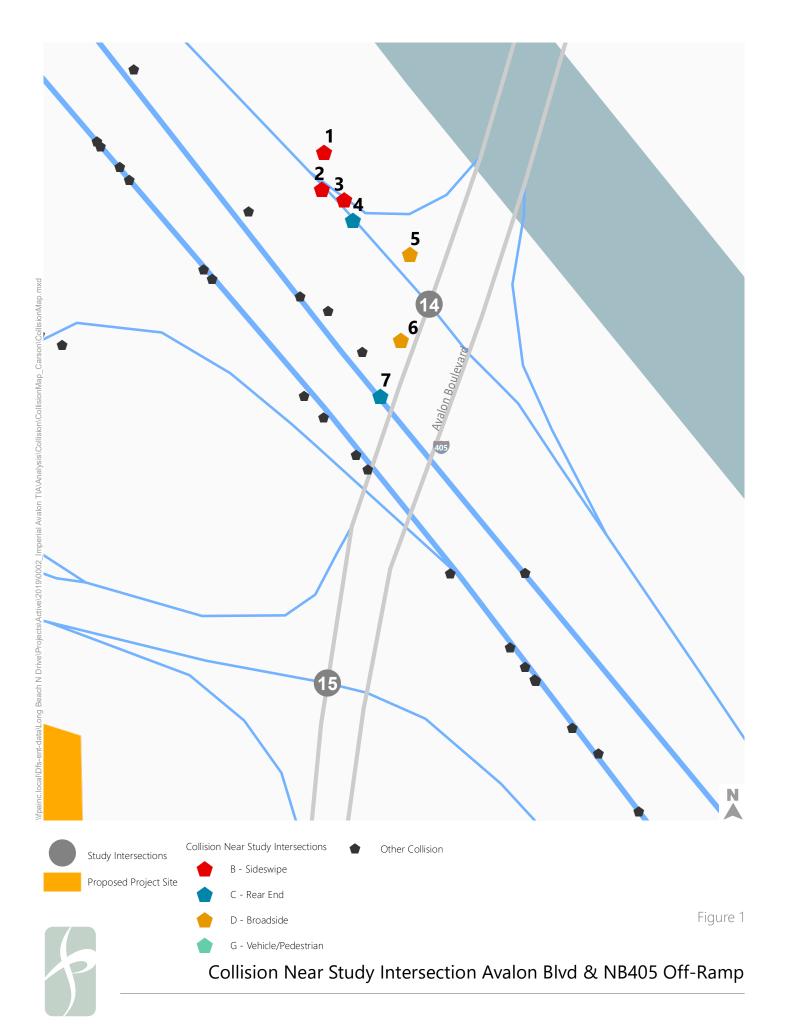
1. Avalon Blvd/Artesia Blvd/NB 91 On-Ramp	2. Central Ave/Artesia Blvd	3. Avalon Blvd/Albertoni St	4. Central Ave/Albertoni St	5. Avalon Blvd/Victoria St	6. Main St/M.L.K. Jr. St	7. Avalon Blvd/M.L.K. Jr. St
(i) (ii) (iii) (ii	Artesia Blvd  Artesia Blvd  Artesia Blvd  Artesia Blvd	Albertoni St  O (0) O (0	Albertoni St  0 (0)  0	0 (0) → 0 (0) 0 (	(0) 0 (0) 1 (3) MLK Jr. Si (0) 0 (0) (0) (0) (0) (0) (0) (0) (0) (	3 (2) (8) (9) (9) (1) (1) (1) (1) (1) (1) (1) (1) (1) (1
8. Main St/I-405 NB Off-Ramp	9. Main St/I-405 SB On-Ramp	10. Main St/Del Amo Blvd	11. Avalon Blvd/Del Amo Blvd	12. Central Ave/Del Amo Blvd	13. Main St/Torrance Blvd	14. Avalon Blvd/I-405 NB Ramps
(a) (b) (c) (c) (d) (d) (d) (d) (e) (d) (e) (e) (e) (e) (e) (e) (e) (e) (e) (e	0 (0) 0 (0) 0 (0) 0 (0) 0 (0)	Del Amo Blvd  0 (0) 0 (0) 0 (0) 0 (0) 0 (0) 0 (0) 0 (0) 0 (0) 0 (0) 0 (0) 0 (0) 0 (0) 0 (0) 0 (0)	Del Amo Blvd  0 (0)  0	© © 0 (0)  Del Amo Bivd  3 (2)  0 (0)  3 (0)	0 (0) 0 (0)	(0) 0 (15 (34) (10) (15 (34) (10) (10) (10) (10) (10) (10) (10) (10
15. Avalon Blvd/I-405 SB Ramps	16. Avalon Blvd/Main Project Driveway	17. Avalon Blvd/Secondary Project Driveway	18. Grace Ave/North Secondary Project Driveway	19. Grace Ave/South Secondary Project Driveway	20. Main St/213th St	21. Grace Ave/213th St
1405 SB Ramps  0 (0) 0 (0) 70 (158) 70 (158)	(G) (O) (O) (O) (O) (O) (O) (O) (O) (O) (O	24 (15) 7 (0) (0) (0) (15) (15) (15) (15) (15) (15) (15) (15	(i) (i) (ii) (iii)	South Secondary Project Driveway  (0) (0) (15)  South Secondary Project Driveway	213th St	2 (4) 1 (1) 0 (0) 1 (24) 1 (24) 1 (3) 1 (3) 1 (3)
22. Avalon Blvd/213th St	23. Wilmington Ave/213th St	24. I-110 SB Ramps/Carson St	25. Figueroa St/Carson St	26. Main St/Carson St	27. Grace Ave/Carson St	28. Avalon Blvd/Carson St
(4) (8) (8) (14) (14) (15) (15) (15) (15) (15) (15) (15) (15	213th St	© 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 (0) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	© (0) © (0) © (0) © (0) Carson St	Carson St	(b) 4 (8) (0) (0) (1) (2) (4) (8) (1) (1) (1) (1) (1) (1) (1) (1) (1) (1
3 (3) 0 (0) 0 (0) 0 (0)	14 (9) 0 (0) 0 (0) 0 (0)	6 (14) == 0 (0)	0 (0) 6 (14) 0 (0) 0 (0)	1 (3) 7 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	8 (18) 0 (0) 0 (0) 0 (0)	0 (0) 0 (0)
29. I-405 SB Ramps/Carson St	30. I-405 NB Ramps/Carson St	31. Figueroa St/l-110 NB Ramps/220th St	32. Avalon Blvd/220th St	33. Avalon Blvd/223rd St		
Carson St 4 (8)  0 (0) 0 (0) 0 (0) 8 (5)	Carson St (0) 0 (0) (0) (0) (0) (0) (0) (0) (0) (	0 (0) 0	220th St (0) (0) (0) (0) (0) (0) (0) (0) (0) (0)	0 (0) 0 (0)		

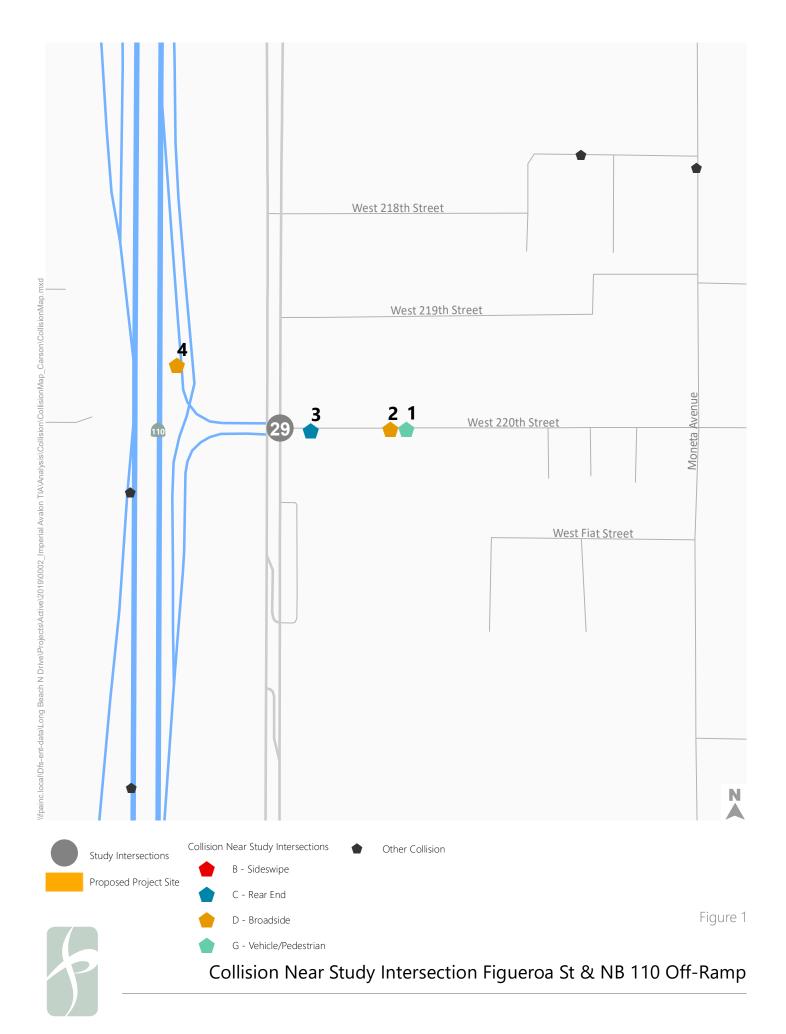




1. Avalon Blvd/Artesia Blvd/NB 91 On-Ramp	2. Central Ave/Artesia Blvd	3. Avalon Blvd/Albertoni St	4. Central Ave/Albertoni St	5. Avalon Blvd/Victoria St	6. Main St/M.L.K. Jr. St	7. Avalon Blvd/M.L.K. Jr. St
1980 (102) 1980 (	(6 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	(1,036) (1,036	1,546 (1,241 Fig. inotadity 15 (1,241 Fig. ino	(0) (2) (2) (2) (372) (2) (2) (372) (372)	357 (480) 244 (625) 217 (228) MLK. Jr. St.	WTF Y 2 202 (226)  4 956 (1,694)
950 (1,308)	238 (320) <u></u> 687 (849) <u></u>	268 (499) 135 (857) 252 (498) 135 (857) 253 (499) 135 (4	367 (312) 186 (850) 1,043 (741) 367 (312) 06 66 80 £2 90 £5	100 (137) - 2,125 (2,212) - 138 (230) - 15 08 4 100 (137) - 100 (1	996 (1,001) +	204 (391) 185 (509) 1757 (506) 185 (509) 185 (509) 1757
8. Main St/I-405 NB Off-Ramp	9. Main St/I-405 SB On-Ramp	10. Main St/Del Amo Blvd	11. Avalon Blvd/Del Amo Blvd	12. Central Ave/Del Amo Blvd	13. Main St/Torrance Blvd	14. Avalon Blvd/I-405 NB Ramps
792 (747) 792 (747) 397 (126) 273 (221)	Man St.	(041) 88 1 1,786 (1,166) 295 (285)	20 (318) (31	(208) (	43 (16) 43 (16) 43 (16) 43 (16) 43 (16) 43 (11) 43 (16)	7515 (515) 672 (543) 674 (543) 777 (331)
40 (18) 4 889 (882)	138 (220) 89 (402) 25 (84) 138 (220) 601 90 E	106 (173) 1,010 (1,784) 1,010 (1,784) 1,010 (205) 1,784 1,78	138 (213) 33 (265) 134 (265) 134 (265) 135 (253) 365 (265) 136 (265) 136 (265) 137 (26	303 (347) <del>- 1</del> 683 (1,300) <del>- 1</del>	235 (414) 19 (64) 235 (552) 20 (652) 20 (652)	796 (622) 7 796 (1,117) —
15. Avalon Blvd/I-405 SB Ramps	16. Avalon Blvd/Main Project Driveway	17. Avalon Blvd/Secondary Project Driveway	18. Grace Ave/North Secondary Project Driveway	19. Grace Ave/South Secondary Project Driveway	20. Main St/213th St	21. Grace Ave/213th St
Availor BMd	140 (215) 1727 (1,757) 140 (118) 140 (118) 140 (118)	Pig (9) (1) (59) (1) (59) (1) (59) (1) (59) (1) (59) (1) (59) (1) (59) (1) (59) (1) (59) (1) (59) (1) (59) (1) (1) (1) (1) (1) (1) (1) (1) (1) (1	Solution Secondary Project Driveway	(E) (0) (0) (0) (19) (0) (0) (0) (19) (0) (0) (0) (0) (0) (0) (0) (0) (0) (0	(1) (10) (10) (10) (10) (10) (10) (10) (	(6) (7) (89) (18) (18) (18) (18) (18) (18) (18) (18
756 (888) 7 (908) 7 (9	222 (137) 0 (0) 40 (23) 8 5 6 8 7 6 9 8 7 6 4 6 8 7 6 4 7 6	24 (15) ~ (£4,1) £8.1	(a) (b) (c) (c) (c) (c) (c) (c) (c) (c) (c) (c	17 (28) 14 (31)	2 (2) → 951 (721) → 167 (262)	19 (51) 323 (224) 4 (56) (6) (6) (7) (7) (8) (8) (1) (7) (8) (1) (1) (1) (1) (1) (1) (1) (1) (1) (1
22. Avalon Blvd/213th St	23. Wilmington Ave/213th St	24. I-110 SB Ramps/Carson St	25. Figueroa St/Carson St	26. Main St/Carson St	27. Grace Ave/Carson St	28. Avalon Blvd/Carson St
75 (91) 75 (91) 75 (91) 75 (91) 76 (118) 76 (118)	24 (162) (162) (162) (162) (1631) (16 (37) (37) (37) (46) (46) (46) (47) (47) (47) (47) (48) (48) (49)	(1,087) (1,087) (1,087) (1,087) (1,087) (1,087) (1,087)	(105) Carson St	(Carson St. Carson St.	(117) (1	172 (214) 172 (214) 172 (214) 172 (214) 172 (214) 172 (214) 173 (352) 174 (352) 175 (352) 175 (352) 176 (352) 177 (352)
282 (285) 161 (272) 72 (88) 161 (272) 72 (88) 17 (121) 19 (121) 19 (121) 19 (121)	71 (49) 00 (20) 120 (90) 120 (90)	1,190 (1,792) 136 (263)	505 (574) 635 (1,204) 505 (574) 114 (141) 7 (2,92) (2,92) (2,92) (3,02) (3,02) (3,02) (4,02) (4,02) (5,02) (6,02) (7,02) (8,02) (8,02) (9,02) (1,02)	166 (258) 75 (258) 73 (132) 73 (132) 75 (258) 75	847 (1,142) 32 (44) 848 (97) 7 (0 (6 (9 (9 (9 (9 (9 (9 (9 (9 (9 (9 (9 (9 (9	740 (901) 74 (108) 74 (108) 74 (108)
29. I-405 SB Ramps/Carson St	30. I-405 NB Ramps/Carson St	31. Figueroa St/l-110 NB Ramps/220th St	32. Avalon Blvd/220th St	33. Avalon Blvd/223rd St		
1,380 (1,268) 76 (74)	(25) 7 (12) 81 (20) (20) (20) (20) (20) (20) (20) (20)	144 (73) 100 (54)	7200th 28.  2500th 26.  2500th	253rd 8t 253		
5 (7) 760 (1,300) 688 (1,052) 760 (1,300) 760 (1,3	38 (30) 38 (30) 00 - 4 00 (146) 00 (20 (30 (30 (30 (30 (30 (30 (30 (30 (30 (3	108 (94) 219 (171) 81 (191) (277) (277) (277) (277) (277)	159 (237) 116 (125) 59 (29) 25 (27) 15 (29) 15 (27) 15 (27)	312 (296) 7 741 (1,079) 37 127 (192) 7 7 8 8 8 18 127 (192) 7 7 8 6 8 18 7 8 18 8 18 7 127 (192) 7		

APPENDIX: Collision Data





Collision Summary for Intersection #14 and #31. From 2016/01/01-2020/12/31

Source: TIMS. SWITRS GIS Map. https://tims.berkeley.edu/tools/gismap/

Intersection Avalon Blvd & NB405 Off-Ramp

Collision Number	Case ID	Date	Location (Intersection)	Dist.&Dir. From intersection	Collision Severity	Injuries	Type of Collision	Party Number	Party Type	Statewide Vehicle Type	At Fault	Party Direction	Movement Preceding Collision	
1	91329649	10/20/2020	I-405 N/b From Avalon Blvd. &	150.00 ft North	4 - Injury	0 Killed & 3 Injured	B - Sideswipe	1	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	Yes	North	P - Merging	
1	31323043	10/20/2020	Avalon Blvd.	130.00 11 1001111	(Complaint of	o killed & 3 ilijured	B - Sideswipe	2	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	No	North	B - Proceeding Straight	
2	90940604	2/27/2019	I-405 N/b From Avalon Blvd. &	50.00 ft West	4 - Injury	0 Killed & 3 Injured	B - Sideswipe	1	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	No	North	D - Making Right Turn	
2	90940604	2/2//2019	Avalon Blvd.	50.00 it west	(Complaint of	o killed & 3 ilijured	b - Sideswipe	2	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	No	North	E - Making Left Turn	
2	90502922	7/7/2017	I-405 N/b From Avalon Blvd. &	105.00 ft North	4 - Injury	0 Killed & 1 Injured	B - Sideswipe	1	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	Yes	North	P - Merging	
3	90302922	7/7/2017	Avalon Blvd.	105.00 It NOITH	(Complaint of	o killed & 1 illjured	U Killed & 1 Injured B - Sideswipe	2	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	No	North	B - Proceeding Straight	
			I-405 Nb From Avalon Blvd &	4 - Injury		4 - Injury			1	1 - Driver (including Hit and Run)	Not Stated	Yes	North	P - Merging
4	90411221	3/4/2017	Avalon Blvd.	30.00 ft North	(Complaint of	0 Killed & 2 Injured	C - Rear End	2	1 - Driver (including Hit and Run)	D - Pickup or Panel Truck	No	North	B - Proceeding Straight	
			Avaion biva.		Pain)			3	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	No	North	B - Proceeding Straight	
-	91179606	12/16/2019	I-405 N/b From Avalon Blvd. &	85.00 ft North	4 - Injury	0 Killed & 1 Injured	D - Broadside	1	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	Yes	North	L - Entering Traffic	
3	911/9606	12/10/2019	Avalon Blvd.	85.00 IL NOITH	(Complaint of	o killed & 1 illjured	D - Broauside	2	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	No	North	B - Proceeding Straight	
6	8785261	12/15/2018	Avalon Bl & Dominguez St	800.00 ft South	4 - Injury	0 Killed & 1 Injured	D - Broadside	1	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	Yes	South	B - Proceeding Straight	
0	8783201	12/15/2016	Avaiori Bi & Dorninguez 3t	800.00 It 30util	(Complaint of	o killed & 1 illjured	D - Broauside	2	1 - Driver (including Hit and Run)	I - Other Bus	No	North	E - Making Left Turn	
7	8403361	6/10/2017	Avalon Bl & Dominguez St	2500.00 ft South	4 - Injury	0 Killed & 1 Injured	C - Rear End	1	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	Yes	North	B - Proceeding Straight	
	0403301	0/10/2017	Avaion bi & Donninguez 3t	2300.00 It 30util	(Complaint of	o killed & 1 Illjured	C - Nedl Ellu	2	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	No	North	A - Stopped	

Intersection Figueroa St & NB 110 Off-Ramp

Collision				Dist.&Dir. From	Collision			Partv				Party	Movement Preceding				
Number	Case ID	Date & Time	Location (Intersection)	intersection	Severity	Injuries		Number	Party Type	Statewide Vehicle Type	At Fault	Direction	Collision				
1	8081556	5/17/2016	220th St & Figueroa St	400.00 ft East	3 - Injury (Other	0 Killed & 1 Injured G	0 Killed & 1 Injured	G - Vahisla/Radastrian	1	2 - Pedestrian	N - Pedestrian	Yes	South	R - Other			
1	8081330	3/17/2010	220til St & Liguel 0a St	400.00 It Last	Visible)			o killed & 1 ilijuled	injured G - Venicle/Fedestrian	2	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	No	East	B - Proceeding Straight		
,	8400963	6/12/2017	220th St & Figueroa St	350.00 ft East	4 - Injury	4 - Injury	0 Killed & 1 Injured	D - Broadside	1	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	Yes	-	L - Entering Traffic			
2	8400963	6/12/2017	220til St & Figueroa St	550.00 IL EdSL	(Complaint of	U Killed & 1 Injured		D - Bloduside	2	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	No	East	B - Proceeding Straight			
,	8916498	8/8/2019	220th St & Figueroa St	100.00 ft East	4 - Injury		0 Killed & 1 Injured	0 Kills of 0 1 Indicated	0 Kills d 0 1 Isis	0 Kills 4 0 4 Ississas	& 1 Injured C - Rear End	1	1 - Driver (including Hit and Run)	C - Motorcycle/Scooter	Yes	East	C - Ran Off Road
3	6910496	0/0/2019	220til St & Figueroa St	100.00 It East	(Complaint of			C - Real Ellu	2	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	No	East	F - Making U-Turn			
4	90380263	1/18/2017	I-110 N/b From 220th St. & 220th	150.00 ft South	4 - Injury	0 Killed & 1 Injured	D - Broadside	1	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	Yes	North	B - Proceeding Straight				
4	90300203	1/10/2017	St.	150.00 it South	(Complaint of	o killeu & 1 Injured	D - Di Dadside	2	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	No	North	B - Proceeding Straight				

APPENDIX: Signal Timing Sheets

NB Avalon Blvd (2 ped)

NBLT Avalon Blvd SB Avalon Blvd

8) NB off Ramp

System:

1) p 2)

H 3) A 4)

S 5)

7)

O A) V B) E C) R D) L E) P F)

Master At: Here

District: 07

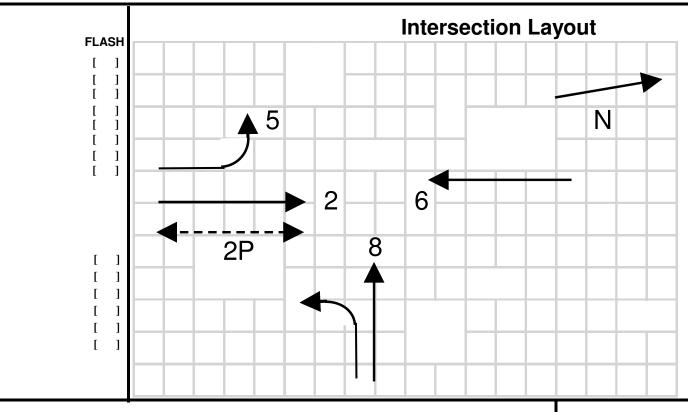
I/C: NB-SB

Designed By: HD Installed By: HD

Service Info:

 Timing Change:
 Date Start:
 Date End:
 Designed:
 Installed:

 11/25/2020
 11/12/2020
 11/12/2020
 11/25/2020



**Comments and Notes:** 

#### **RAM Checksum**

Page 2: 3B49 Page 8: F1FF
Page 3: EA11 Page 9: D2FD
Page 4: 8EA7 Page 10: D42B
Page 5: 191A Page 11: 675C
Page 6: 191A Page 12: 1FB3
Page 7: 12D9 Page 13: 86F7

Post Mile: 11.27 Printed: 5/11/2021

. 2 . . . 6 . .

#### Location: R405 NB @ Avalon Blvd

Cabinet ( 9-3 )					
332					
Configuration					
CALTRANS					

Vehicle Min

Vehicle Max Pedestrian

Bicycle

Phase Recalls (2-1-1-2)

Phases ( 2-1-1-1 )							
Permitted	.256.8						
Restricted							

### **CONFIGURATION PHASE FLAGS**

Dhasa Lasks (	0.1.1.0.\					
Phase Locks (2-1-1-3)						
Red	5					
Yellow	.26					
Force/Max						

Phase Features ( 2-1-1-4 )						
Double Entry						
Rest In Walk						
Rest In Red						
Walk 2						
Max Green 2						
Max Green 3						

Startup ( 2-1-1-5 )						
First Green Phases	. 2 6					
Yellow Start Phases	8					
Vehicle Calls	.256.8					
Pedestrian Calls	. 2					
Yellow Start Overlaps						
Startup All-Red	6.0					

Cá	all To Phase ( 2-1-2-1 )		Omit On Green
1		1	
2		2	
3		3	
4		4	
5		5	
6		6	
7		7	
8		8	

Flashing Colors ( 2-1-2-2 )							
Yellow Flash Phases							
Yellow Flash Overlaps							
Flash In Red Phases							
Flash In Red Overlaps							

Special Operation ( 2-1-2-3 )							
Single Exit Phase							
Driveway Signal Phases							
Driveway Signal Overlaps							
Leading Ped Phases							

Protected Permissive ( 2-1	-2-4 )
Protected Permissive	

Pede	Pedestrian (2-1-3)							
P1								
P2	.2							
P3								
P4	4							
P5								
P6	6							
P7								
P8	8							

Overlap ( 2-1-4 )												
Overlap	Parent	Omit	No Start	Not								
Α												
В												
С												
D												
E												
F												

P
Н
A
S
Ε
T
M
N
G

Overlap (2-4)

Green

Yellow

Red

Phase ( 2-2 )	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Walk 1	0	7	0	10	0	0	0	0
Flash Don't Walk	0	10	0	10	0	0	0	0
Minimum Green	10	10	10	10	10	10	10	10
Det Limit	10	0	10	10	0	0	10	0
Max Initial	10	20	10	10	0	20	10	0
Max Green 1	50	40	50	50	20	40	50	25
Max Green 2	50	40	50	50	20	40	50	25
Max Green 3	50	40	50	50	20	40	50	25
Extension	5.0	4.0	5.0	5.0	2.5	4.0	5.0	4.0
Maximum Gap	5.0	5.0	5.0	5.0	2.5	5.0	5.0	5.0
Minimum Gap	5.0	3.0	5.0	5.0	2.5	3.0	5.0	3.0
Add Per Vehicle	1.0	2.0	1.0	1.0	0.0	2.0	1.0	0.0
Reduce Gap By	0.0	0.1	0.0	0.0	0.0	0.1	0.0	0.0
Reduce Every	1.0	1.0	1.0	1.0	0.0	1.0	1.0	0.0
Yellow	5.0	4.4	5.0	5.0	3.7	4.4	5.0	4.1
All-Red	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Ped/Bike (2-3)	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Walk 2	0	0	0	0	0	0	0	0
Delay/Early Walk	0	0	0	0	0	0	0	0
Solid Don't Walk	0	0	0	0	0	0	0	0
Bike Green	0	0	0	0	0	0	0	0
Bike All-Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

#### **OVERLAP TIMING**

0.0

5.0

0.0

С

В

0.0

5.0

0.0

Α

0.0

5.0

0.0

#### 

**Max 2 Extension** 

Max/Gap Οι	ıt ( 2-7 )
Max Cnt	0
Gap Cnt	0

Post Mile: 11.27 PAGE 3 CHECKSUM: EA11 Printed: 5/11/2021

0.0

5.0

0.0

Е

0.0

5.0

0.0

#### Local Plan 1...9 (7-1) TIMING DATA COORDINATION Master Timer Sync (7-A) **Enable in Plans** Green Factors or Press [F] to Select Force-Off [ Offsets ] 1-9 Cycle | Multi Lag Gap C -1--2--3--4--5--7--8-. . . . . . . . . Plan 1 Green Factor 80 20 46 21 20 22 11-19 . . . . . . . . 21-29 Plan 2 Green Factor 80 22 20 46 21 20 . . . . . . . . **Master Sub Master** Plan 3 Green Factor 80 22 20 21 20 46 Input Plan 4 Green Factor Output Plan 5 Green Factor . . . . . . . . FREE PLAN PHASE FLAGS Plan 6 Green Factor (7-E) Free Omit Lag Plan 7 Green Factor . . . . . . . . .2.4.6.8 . . . . . . . . Veh Min Veh Max Plan 8 Green Factor . 2 . . . 6 . . Plan 9 Green Factor Ped Bike Local Plan 1...9 (7-1) PHASE FLAGS Cond **Cond Grn** 10 Lag Veh Max Ped Bike Sync Hold Omit Veh Min Plan 1 .2.4.6.8 . 2 . . . 6 . . . 2 . MANUAL COMMANDS Plan 2 Manual Plan (4-1) Plan: 1-29 .2.4.6.8 . 2 . . . 6 . . . 2 . . . . . 254 = Flash OffSet Plan Plan 3 .2.4.6.8 .2...6.. . 2 . 255 = Free Α Plan 4 Offset A, B, or C \_ \_ \_ \_ \_ \_ \_ \_ \_ . . . . . . . . Special Function Override (4-2) Plan 5 . . . . . . . . Control # Control Plan 6 . **NORMAL** 3 **NORMAL** Plan 7 **NORMAL NORMAL** Plan 8 . **Detector Reset** (4-3)Plan 9 . Local Manual (4-4) **OFF**

### Local Plan 11...19 (7-2) TIMING DATA

### **COORDINATION**

[ Offsets ]

Green Factors or Press [F] to Select Force-Off

		[ Onsets ] Green actions of Tress [1] to detect 1 order on													
		Cycle	Multi	Lag Gap	Α	В	С	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Plan 11	Green Factor														
Plan 12	Green Factor														
Plan 13	Green Factor														
Plan 14	Green Factor														
Plan 15	Green Factor														
Plan 16	Green Factor														
Plan 17	Green Factor														
Plan 18	Green Factor														
Plan 19	Green Factor														

# Local Plan 11...19 (7-2) PHASE FLAGS

	Lag	Sync	Hold	Omit	Veh Min	Veh Max	Ped	Bike
Plan 11								
Plan 12								
Plan 13								
Plan 14								
Plan 15								
Plan 16								
Plan 17								
Plan 18								
Plan 19								

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# Local Plan 21...29 (7-3) TIMING DATA

#### **COORDINATION**

[ Offsets ]

Green Factors or Press [F] to Select Force-Off

		[ Griscis ] Green actors on Tress [1] to detect 1 dide on													
		Cycle	Multi	Lag Gap	Α	В	С	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Plan 21	Green Factor														
Plan 22	Green Factor														
Plan 23	Green Factor														
Plan 24	Green Factor														
Plan 25	Green Factor														
Plan 26	Green Factor														
Plan 27	Green Factor														
Plan 28	Green Factor														
Plan 29	Green Factor														
															Ш

# Local Plan 21...29 (7-3) PHASE FLAGS

	Lag	Sync	Hold	Omit	Veh Min	Veh Max	Ped	Bike
Plan 21								
Plan 22								
Plan 23								
Plan 24								
Plan 25								
Plan 26								
Plan 27								
Plan 28								
Plan 29								

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### **DETECTORS**

Dete	ector Attributes (5-1)		Slot	Detector Configuration (5-2)					
Det		Phases	Lock		Det	Delay	Extend	Recall	Port
1	COUNT+CALL+EXTEND	1	NO	I1U	1			10	3.2
2	COUNT+CALL+EXTEND	1	NO	I1L	2			10	7.2
3	COUNT+CALL+EXTEND	. 2	NO	I2U	3			10	1.1
4	COUNT+CALL+EXTEND	. 2	NO	I2L	4			10	1.5
5	COUNT+CALL+EXTEND	. 2	NO	I3U	5			10	4.5
6	CALL+EXTEND	. 2	NO	I3L	6			10	6.2
7	LIMITED	. 2	NO	I4U	7			10	2.1
8	COUNT+CALL+EXTEND	. 2	NO	I4L	8			10	7.4
9	COUNT+CALL+EXTEND	3	NO	I5U	9			10	3.4
10	COUNT+CALL+EXTEND	3	NO	I5L	10			10	7.6
11	COUNT+CALL+EXTEND	4	NO	I6U	11			10	1.3
	COUNT+CALL+EXTEND	4	NO	I6L	12			10	1.7
13	COUNT+CALL+EXTEND	4	NO	I7U	13			10	4.7
14	CALL+EXTEND	4	NO	17L	14			10	6.4
	LIMITED	4	NO	I8U	15			10	2.3
	COUNT+CALL+EXTEND	4	NO	ISL	16			10	7.8
17	COUNT+CALL+EXTEND	1	NO	I9U	17			10	3.6
	COUNT+CALL+EXTEND	3	NO	19U	18			10	3.8
19	COUNT+CALL+EXTEND	. 2	NO	I10U	19			10	4.1
	COUNT+CALL+EXTEND	4	NO	110U 110L	20			10	4.2
	COUNT+CALL+EXTEND	5	NO	J1U	21			10	3.1
	COUNT+CALL+EXTEND	5	NO	J1L	22			10	7.1
	COUNT+CALL+EXTEND	6	NO	J2U	23			10	1.2
	COUNT+CALL+EXTEND	6	NO	J2L	24			10	1.6
	COUNT+CALL+EXTEND	6	NO	J3U	25			10	4.6
	CALL+EXTEND	6	NO	J3L	26			10	6.3
	LIMITED	6	NO	J4U	27			10	2.2
	COUNT+CALL+EXTEND	6	NO	J4L	28			10	7.3
	COUNT+CALL+EXTEND	7.	NO	J5U	29			10	3.3
	COUNT+CALL+EXTEND	7.	NO	J5L	30			10	7.5
	COUNT+CALL+EXTEND	8	NO	J6U	31			10	1.4
	COUNT+CALL+EXTEND	8	NO	J6L	32			10	1.8
	COUNT+CALL+EXTEND	8	NO	J7U	33			10	4.8
34		8	NO	J7L	34			10	6.5
	LIMITED	8	NO	J8U	35			10	2.4
	COUNT+CALL+EXTEND	8	NO	J8L	36			10	7.7
	COUNT+CALL+EXTEND	5	NO	J9U	37			10	3.5
	COUNT+CALL+EXTEND	7.	NO	J9L	38			10	3.7
	COUNT+CALL+EXTEND	6	NO	<b>J10U</b>	39			10	4.3
	COUNT+CALL+EXTEND	8	NO	J10L	40			10	4.4
	PEDESTRIAN	. 2	NO	I12U	41			10	5.1
42	PEDESTRIAN	4	NO	I12L	42			10	5.3
43	PEDESTRIAN	6	NO	I13U	43			10	5.2
44	PEDESTRIAN	8	NO	I13L	44			10	5.4
				_		· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·	

Maximum On Time	Failure Times(5-3)	Minutes
Fail Reset Time	Maximum On Time	
i dii itooot iiiio	Fail Reset Time	

Failure Override (5-4)							
Detectors 1-8							
Detectors 9-16							
Detectors17-24							
Detectors 25-32							
Detectors 33-40							
Detectors 41-44							

System Detector Assignment (5-5)												
Sys Det	1	2	3	4	5	6	7	8				
Det Nu												
Sys Det	9	10	11	12	13	14	15	16				
Det Nu												

CIC Operation (5-6-1	)
Enable in Plans	

CIC Values (5-6-2)	Volume	Occupancy	Demand
Smoothing	0.66	0.66	0.66
Multiplier	4.0	0.33	
Exponent	0.50	1.00	

	Detec	Detector-to-Phase Assignment (5-6-3)											
Sys Det	1	1 2 3 4 5 6 7											
Phase													
Sys Det	9	10	11	12	13	14	15	16					
Phase													

# **Input File Port-Bit Assignments**

332 Cabinet - For Reference Only

	1	2	3	4	5	6	7	8	9	10	11	12	13	14
I-	3.2	1.1	4.5	2.1	3.4	1.3	4.7	2.3	3.6	4.1	6.6	5.1	5.2	<b>6.7</b>
	7.2	1.5	6.2	7.4	7.6	1.7	6.4	7.8	3.8	4.2	2.7	5.3	5.4	6.8
J-	3.1	1.2	4.6	2.2	3.3	1.4	4.8	2.4	3.5	4.3	2.8	5.5	5.6	2.5
	7.1	1.6	6.3	7.3	7.5	1.8	6.5	7.7	3.7	4.4	6.1	5.7	5.8	2.6

# TOD SCHEDULE

Location: R405 NB @ Avalon Blvd

Table 1	Table 1 (8-2-1) Tal		Table 2	Table 2 (8-2-2)		Table 3	Table 3 (8-2-3)		Та	Table 4 (8-2-4)		Table 5 (8-2-5)			Table 6 (8-2-6)			
Time	Plan	os	Time	Plan	os	Time	Plan	os	Т	ime	Plan	os	Time	Plan	os	Time	Plan	os
0630	2	Α	0900	1	Α			Α				Α			Α			Α
0900	1	Α	1800	255	Α			Α				Α			Α			Α
1500	3	Α			Α			Α				Α			Α			Α
1800	1	Α			Α			Α				Α			Α			Α
2000	255	Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α

### **WEEKDAY ASSIGNMENT**

Weekday Table Assignments (8-2-7)											
Mon Tue Wed Thu Fri Sat Sur											
1	1	1	1	1	2	2					

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#### **HOLIDAY TABLES**

Flo	Floating Holiday Table (8-2-8)										
#	Mnth	Week	DOW	Table							
1											
2											
3											
4											
5											
6											
7											
8											
9											
10											
11											
12											
13											
14											
15											
16											

#	Mnth	Day	DOW	Table
1				
2				
3				
4				
5				
6				
7				
8				
9				
10				
11				
12				
13				
14				
15				
16				

Daylight Saving (8-1)					
Enabled	YES	Month	Sunday		
	Start		2nd		
	End	NOV	1st		

Solar Clock Data (8-4)					
North Latitude 34					
West Longitude	118				
Local Time Zone	8				

Sabbatical Clock (8-5)					
Hebrew	Ped Recall				
Sabbath					
Holiday					

#### **TOD FUNCTIONS**

ТО	TOD Functions (8-3)									
#	Start	End	DOW	Action	Phases					
1			• • • • • •							
2			• • • • • •							
3			• • • • • •							
4			• • • • • •							
5			• • • • • •							
6			• • • • • •							
7			• • • • • •							
8			• • • • • •							
9			• • • • • •							
10			• • • • • •							
11			• • • • • •							
12			• • • • • •							
13										
14										
15										
16			• • • • • •							

**Action Codes:** 

- 0. None
- 1. Permitted
- 2. Restricted
- 4. Veh Min Recall
- 5. Veh Max Recall
- 6. Ped Recall
- \_ \_.. \_ ..
- 7. Bike Recall
- 8. Red Lock
- 9. Yellow Lock
- 10. Force/Max Lock
- 11.Double Entry
- 12. Y-Coord C
- 13. Y-Coord D
- 14. Free
- 15. Flashing
- 16. Walk 2
- 17. Max Green 2

- 18. Max Green 3
- 19. Rest in Walk
- 20. Rest in Red
- 21. Free Lag Phases
- 22. Special Functions
- 23. Truck Preempt
- 24. Conditional Service
- 25. Conditional Service
- 26. Leading Ped
- 27. Traffic Actuated Max 2
- 41. Protected Permissive
- 42. Protected Permissive

Action Code = Phases added to normal setting

100+Action Code = Phases removed

200+Action Code = Phases replaced

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#### **COMMUNICATIONS**

	C2 (6-1-1)	C20 (6-1-2)	C21 (6-1-3)	
Address				
Baud	1200	1200	1200	
Protocol	MASTER	AB3418	AB3418	
Data Bits	8	8	8	
Parity	NONE	NONE	NONE	
Stop Bits	1	1	1	
RTS On Time	20	20	20	
RTS Off Time	20	20	20	
Handshaking	NORMAL	NORMAL	NORMAL	
Access Level	0	0	0	

### **SOFT LOGIC**

So	ft Logic (						
#	Data	OP	Data	OP	Data	OP	Data
1							
2							
3							
4							
5							
6							
7							
8							
9							
10							
11							
12							
13							
14							
15							
16							

<sup>\*</sup>Refer to User's Manual for Data and OP Codes

#### **CALLBACK NUMBERS**

Callback Numbers (6-33)							
Line Out							
Long Distance							
Local Toll							
Delay	10	10	10				
Area Code							
Phone Number							

#### **NETWORK**

Network Parameters (6-4)					
Address					
Protocol	AB3418				
Port	27000				
Туре	STATIC				
<b>Central Access</b>					
Field Access					
ATSPM	OFF				

IP Address	0	0	0	0
Netmask	255	255	255	0
Broadcast	0	0	0	255
Gateway	0	0	0	254

#### **Access Levels:**

**0-Full Access** 

1-Status Only

2-Status, Set Pattern, Time

3-Status, Set Pattern, Time, Manual Plan

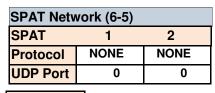
4-Reserved

5-Full Access with No Set Pattern

6-Full Access with No Set Time

7-Full Access with No Set Pattern, Manual Plan

8-Full Access with No Set Time, Pattern, Manual Plan



IP Address 0 . 0 . 0 .

#### **RAILROAD PREEMPTION**

RR 1	Timing ( 3-1-1 )						
•	Clear 1	15					
	Clear 2	5					
	Clear 3						
	Hold						
	Min Gr						
	Delay						
	Exit						
	Ped Clr						

Phase Flags (3-1-2)			Pedestrian Flags (3-1-3)			Overlap Flags (3-1-4)		
Grn Hold	Yel Flash	Red Flash	Walk	Flash DW	Solid DW	Grn Hold	Yel Flash	Red Flash
. 2 5					.2.4.6.8			
. 2 5					.2.4.6.8			
1478				48	.26			

Exit Parameters (3-1-5)							
	Phase Green	Ovrlap Green	Veh Permit/Call	Ped Permit/Call			
	.26		12345678	.2.4.6.8			
'							

(	Configuration (3-1-6)										
	PR	XR	Gate	Isld	APP		Sign	Sign		Max On	Latchin
1										5	NO
2			Valid Inp	uts: 1.	x, 2.x, 3.	x,	4.x, 5.x,	6.x, 7.x	Χ,	8.x x=1	to 8
			Valid Out	puts: 11.	x <u>, 1</u> 2.x, <u>13</u> .	х,	14.x, 15.x,	16.x, 17.	х,	18.x x=1	to 8

RR 2	Timing	( 3-2-1 )
_	Clear 1	15
	Clear 2	5
	Clear 3	
	Hold	
	Min Gr	
	Delay	
	Exit	
	Ped Clr	

	Phase Flags (3-2-2)			Pedestrian Flags (3-2-3)			Overlap Flags (3-2-4)		
	Grn Hold	Yel Flash	Red Flash	Walk	Flash DW	Solid DW	Grn Hold	Yel Flash	Red Flash
	47.					.2.4.6.8			
	47.					.2.4.6.8			
1	1236				. 2 6	48			

Exit Parameters (3-2-5)							
Phase Green Ovrlap Green   Veh Permit   Ped Permit							
.26		12345678	.2.4.6.8				

Configu	ration (3-	-2-6)			_						
PR	XR	Gate	Isld	APP		Sign	Si	ign	Max	On	Latching
									5	)	NO
			-	_	•		-	•	_		

✓ Valid Inputs: 1.x, 2.x, 3.x, 4.x, 5.x, 6.x, 7.x, 8.x x=1 to 8 Valid Outputs: 11.x, 12.x, 13.x, 14.x, 15.x, 16.x, 17.x, 18.x x=1 to 8

### **EMERGENCY VEHICLE PREEMPTION**

EVA	Pro	eempt Tim	ers	Phase Green	Overlap
(3-A)	Delay Clear		Max		Green
		8	30	.25	• • • • •

Port	Latching	Phase Termination
	NO	ADVANCE

EVC	Pro	eempt Tim	ers	Phase Green	Overlap
(3-C)	Delay	Clear	Max		Green
		8	30	16	• • • • •

Port	Latching	Phase Termination
	NO	ADVANCE

EVB	Pre	empt Tim	ers	Phase Green	Overlap
(3-B)	Delay	Clear	Max		Green
		30	30	47.	

Port	Latching	Phase Termination
	NO	ADVANCE

EVD	Preempt Timers  Delay Clear Max		Phase Green	Overlap	
(3-D)	Delay	Clear	Max		Green
		8	30	38	• • • • •

Port	Latching	Phase Termination
	NO	ADVANCE

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INPUTS		7 Wire I/C ( 2-1-5-1 )			
		Input	Port	Input	Port
Enable	NO	RR1		Free	
Max ON		RR2		D2	
Max OFF		RR3		D3	

Manual Control ( 2-1-5-2 )		
Input	Port	
Manual Advance		
Advance Enable		

Cabinet Status ( 2-1-5-3		
Input	Port	
Flash Bus		
Door Ajar		
Flash Sense	6.7	
Stop Time	6.8	

	Special Function (2-1-5-4)		
j	Input	Poi	rt
	1		
	2		
	3		
	4		

Battery Backup ( 2-1-5-5 )			
Port	Operation		
	NORMAL		

Y-Coordination (2-1-5-6		
Port C	Port D	

### **OUTPUTS**

Loadswitch Assignments ( 2-1-6 )							
Α	1	2	22	3	4	24	9
В	5	6	26	7	8	28	10
X	13	14	0	11	12	0	0

**Loadswitch Codes:** 

0 Unused (no output)

1-8 Vehicle 1-8

9-14 Overlap A-F

21-28 Ped 1-8

41-47 Special Functions

41 Protected Permissive Flashing Phase 1

43 Protected Permissive Flashing Phase 3

45 Protected Permissive Flashing Phase 5

47 Protected Permissive Flashing Phase 7

51-57 Special Functions

71-72 Seven Wire I/C

+ middle output of loadswitches 3 and 6 Channel 9 and 10

#### **INTERVAL CONTROL**

Interval	Control
(3-3-1)	Time
Step 1	
Step 2	
Step 3	
Step 4	
Step 5	
Step 6	
Step 7	
Step 8	

Phase Control (3-3-2)				
Hold	Force	Advance		

	Phase Recall (3-3-3)				
	Veh Call	Ped Call	Int Call		
1					

	Phase Permitted (3-3-4)				
Phs Permit		Ped Permit	<b>Ovrlap Permit</b>		

Configu	Configuration (3-3-5)									
Input	Port	Delay	<b>HRI Cross</b>							
1										
2										

HRI

	HRI Configuration (3-4)										
RailRoad	51		WAYSIDE	ATC							
Line		Subnode									
Group		Device									

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#### TRANSIT PRIORITY

Location: R405 NB @ Avalon Blvd

Local Pla	ans (3-E) 19 1119	Early	Green	Inhibit	Phase 1 Minimum					Phase 6		
	_	Green	Extend	Cycles	winimum	wiinimum	Wilnimum	Wiinimum	Wiinimum	Wilnimum	wiinimum	Wiinimum
Plan 1	Green Factor											
Plan 2	Green Factor											
Plan 3	Green Factor											
Plan 4	Green Factor											
Plan 5	Green Factor											
Plan 6	Green Factor											
Plan 7	Green Factor											
Plan 8	Green Factor											
Plan 9	Green Factor											
								· · · · · · · · · · · · · · · · · · ·				
Plan 11	Green Factor											
Plan 12	Green Factor											
Plan 13	Green Factor											
Plan 14	Green Factor											
Plan 15	Green Factor											
Plan 16	Green Factor											
Plan 17	Green Factor											
Plan 18	Green Factor											
Plan 19	Green Factor											

Transit Prio	rity Configuration		Indicator Output				
Enable in P	lans	Input	Type	Stop	Go		
Plan 1-9		0.0	NONE	0	0		
Plan 11-19		0.0	NONE	0	0		

Queue Jump (3-E-B)								
Grn Hold	Hold Phase							

Free Plans (3-E-E)							
Max Grn Hold Hold Phase							

Access Utilities (9-5)							
Password	***						
Timeout	30						

### YELLOW YIELD COORDINATION

									-Offs							
Y-Coord Plans (7-C,D)	Long Grn	No Grn	Offset	Perm	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-	Coord	Lag	Min Recall	Restricted
Plan C													.26	.2.4.6.8		
Plan D													. 2 6	.2.4.6.8		

### TRUCK PRIORITY

Truck Priority (3-F)	Passage	CarryOver	Clearance	Next Priority	Phase Green				Sign Output		
						0.0	0.0	0.0	0	0.0	0

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**CALTRANS** 

**C8 PROGRAM** 

Version 3.0

Model 170 Controller Traffic Design Branch District 7 LOCATION: S/B RTE. 405 FWY @ AVALON

CO: L.A.

RTE: 405 P.M.: 11.22

1 OF 6

-COI	$MTD \cap$		)E "F"
CO		L COL	/C

	1				PHASE 7	TIMING						PRE-			F		
	INTERVAL	1	2	3	4	5	6	7	8	С	D	E	FLAGS	1 2	3 4 5	6 7	8
0	WALK		7		6		0		0	GAP SET	TB SEL	1 EV SEL C	FZ PERMIT	Х	Χ	Х	x 0
1	DON'T WALK		13		18		0		0	MAX SET	TB HR	RR1 CLR	RED LOCK		Х		1
2	MIN GREEN		25		24		25		24	C - SERV	TB MIN	EVA DLY	RED/YEL LOCK	Х		Х	2
3	TYPE 3 DET		0		0		0		0		PED SEL	1 CLR	VEH RECALL	Х		Х	3
4	ADDED / ACT		0.0		0.0		0.0		0.0		7W SEL	EVB DLY	PED RECALL				4
5	PASSAGE		5.0		3.0		5.0		5.0		PERM SEL	EVB CLR	PED PHASES	х	х	Х	x 5
6	MAX GAP		5.0		3.0		5.0		5.0			EVC DLY	A ARROW				6
7	MIN GAP		5.0		3.0		5.0		5.0			EVC CLR	B ARROW				7
8	MAX EXT 1		50		25		50		50		OFF SEEK	1 DLY	DOUBLE ENTRY		Х		x 8
9	MAX EXT 2		50		25		50		50			EVD CLR	MAX 2				9
Α	MAX EXT 3		50		25		50		50	OLA GRN		EV MAX	LAG PHASES	OBSEF	RVATIO	N ONL	Y <b>A</b>
В										OLB GRN		RR2 CLR	RED REST				В
С	REDUCE BY		0.0		0.0		0.0		0.0	OLC GRN		EV CLR TMR	REST IN WALK				С
D	" EVERY		0.0		0.0		0.0		0.0	OLD GRN		EV DLY TMR					D
E	YELLOW		4.4		4.1		4.4		4.1	i	i	RR CLR TMR	YEL START-UP		Х		х Е
F	ALL RED		1.0		1.0		1.0		1.0	RAM ADD		EV MAX TMF	FIRST	Х		Χ	F
	DIRECTION		NB-SB		OFF-RAMP FB		DUMMY		DUMMY				CONTR	ROL (	CODE	: "F	10

12

04

03

HD

NOTES AND REMARKS:

DATE

BY

06

18

98

CB

06

18

98

СВ

О	VERLAP PHASES
Α	
В	
С	
D	

12

04

03

HD

DATE START:

DATE SUPERSEDED:

6/18/98

9 YEAR
MONTH
B DAY OF MO
C LONG FAIL DAY OF WK
D SHORT FAIL HOUR
E MAX-VAR-INIT 30 MINUTE
F RED REVERT 5.0 SECOND
FILENAME: 4051122.XLS

9

CLK RST

0

0

LOCATION: S/B RTE. 405 FWY @ AVALON

CO: L.A.

RTE: 405

P.M.: 11.22 2 OF 6

		ATION .									C	ТИС	ROL	_ COI	DE	"C"		L.A.	1112. 400						
				COI	NTR	OL F	PLAN	IS (N	IAXIM	UM OF	9 PL	ANS)					Y-CORD "C"	Y-CORD "D"				FL	AGS		
	0		1												Α	В	С	D	E	F	1 2 3	4 !	5 6	7 8	
0	LOC ADD	CYCLE LENGTH	62												OCAL IMER	MASTER TIMER	RAM TEST		TB LAG	FREE	X	X	X	X	0
1	MANUAL CP	FZ1 GRN FCTR												FZ	Z1 FO	PERM FZ1	INITIAL FLASH		GAP CP1 LAC	CP1	X	X	X	X	1
2	MASTER CP													FZ	Z2 FO		DOWN LOAD <b>1</b>		GAP CP2 LAC	CP2	Х	Х	Х	Х	2
3	CURRENT CP	FZ3 GRN FCTR												FZ	Z3 FO	PERM FZ3			GAP CP3 LAC	CP3	Х	Х	Х	Х	3
4	LAST CP	FZ4 GRN FCTR	25											FZ	Z4 FO	PERM FZ4	PERM TIME	PERM TIME	GAP CP4 LAC	CP4	Х	Х	Х	Х	4
5	FLASH	FZ5 GRN FCTR												FZ	Z5 FO	PERM FZ5	LAG OFS	LAG OFS	CP5	CP5	Х	Х	Х	Х	5
6	CURRENT OFS													FZ	Z6 FO		FORCE OFF	FORCE OFF	GAP CP6 LAC	CP6	Х	Х	Х	Х	6
7	TRANS. CP	FZ7 GRN FCTR												FZ	Z7 FO	PERM FZ7	LONG GRN	LONG GRN	GAP CP7	CP7	Х	Х	Х	Х	7
8	8 KEY BANK	FZ8 GRN FCTR	25											FZ	Z8 FO	PERM FZ8	NO GRN	NO GRN	GAP CP8	CP8	X	X	X	X	8
9	9 KEY BANK	MULTI CYCLE															PERM TIMER	PERM TIMER	GAP CP9 LAC	CP9	X	X	X	Х	9
Α		OFFSET A	0												PRES OFS	DES OFS	YC OFS	YC OFS	YC	AG C					A
В		OFFSET B															OFS TIMER	OFS TIMER	YC	AG D					В
С		OFFSET C															LAG GRN TIMER	LAG GRN TIMER	COC	RD FZ	X		X		C
D	MANUAL OFS	FZ3 EXTENSION															FO TIMER	FO TIMER	Н	OLD	OBSER	VATIO	10 NC	<b>NLY</b>	D
Е		FZ7 EXTENSION															LONG GRN TIMER	LONG GRN TIMER	NE.	(T FZ	OBSER	VATIO	10 NC	<b>NLY</b>	Ε
F		OFS INTERRUPT												LO	C CYC	MAS CYC	NO GRN TIMER	NO GRN TIMER	FC	RCE	OBSER	VATIO			F
	DIRECTION PROGRES	N OF SION	AVG													LOCAL N			DATE 6/18/98 DATE SU	STAR PERSE		HOLIWS	1 3	X X	NO
	ВҮ		RS	+												SYSTEM: MASTE		-AVALON E				λς. Δ			OCATION

BY
NOTES AND REMARKS:

LOCATION: S/B RTE. 405 FWY @ AVALON CO: L.A. RTE: 405 P.M.: 11.22 3 OF 6

						CO	NT	RO	L (	CODE "E"	(TH	IS P	AGE	E @	F-C	-F =	0)		
			Ε										F						
	FUNCTION			PH	ASI	E FL	.AG			FUNCTION			PH	ASE	E FL	.AG			
	TONOTION	1	2	3	4	5	6	7	8	TONOTION	1	2	3	4	5	6	7	8	
0										C. SERV. CODE 4									0
1										C. SERV. CODE 5									1
2										Y CORD RECALL C									2
3										Y CORD RECALL D									3
5										2 PED OUTPUT		Х							5
6										6 PED OUTPUT						Х			6
7										4 PED OUTPUT				Х					7
8										8 PED OUTPUT								Х	8
Α	OVERLAP "A" NOT									OVERLAP "A" ON									Α
В	OVERLAP "B" NOT									OVERLAP "B" ON									В
С	OVERLAP "C" NOT									OVERLAP "C" ON									С
D	OVERLAP "D" NOT									OVERLAP "D" ON									D

NOTES	AND	REMARKS:	

NOTE: IF THIS IS THE MASTER
FOR THE SYSTEM, ENTER NO.
OF SLAVES IN RAM LOCATION:
D-0-0 =

DATE START:
6/18/98
DATE SUPERSEDED:

														CO	NTROL COL	DE	"D'	•						
			Α	В	С	D			PH	ASE	E FL	.AG			E			PH	ASI	E FL	_AG	İ		F PHASE FLAG
						MAX RECALL	1	2	3	4	5	6	7	8	MIN RECALL	1	2	3	4	5	6	7	8	PED RECALL 1 2 3 4 5 6 7 8
1						CP1									CP1									CP1 1
2						CP2									CP2									CP2 2
3						CP3									CP3									CP3 3
4						CP4									CP4									CP4 4
5						CP5									CP5									CP5 5
6						CP6									CP6									CP6 6
7						CP7									CP7									CP7 7
8						CP8									CP8									CP8 8
9						CP9									CP9									CP9 9
Α																								TOD MAX RECALL1 A
В																								TOD MAX RECALL2
С																								С
D																								D
Ε	LAST	PWR FAIL	НН	MM	DD										C8 VER	SION	I NU	MBE	R					E
F	LAS	T FLASH	НН	MM	DD										LITHIUM BATTERY	CON	DITIC	N 84	=BAI	D 85	=GO0	D		F

**NOTES AND REMARKS** 

LOCATION:

CO: <i>L.A.</i>		
'()· / A		

CC	ONTF	ROL	COD	E "9"	•			(	C-O-	9 = 0	OR	1)	C	ONTF	ROL (	COD	E ''9'	•					(C-C	)-9 =	2)
	HR	MIN	СР	OFS		S	М	Т	W	Т	F	S		HR	MIN	СР	OFS		S	М	Т	W	Т	F	S
	(HH)	(MM)				1	2	3	4	5	6	7		(HH)	(MM)				1	2	3	4	5	6	7
0	05	00	1	Α	Е	X	X	X	X	Χ	Х	X	0					Е							
1	22	00	Е		Ε	Х	Х	Х	X	Х	X	Х	1					Ε							
2					Е								2					Е							
3					Е								3					Е							
4					Е								4					Е							
5					Е								5					Е							
6					Е								6					Е							
7					Е								7					Е							
8					Е								8					Е							
9					Е								9					Е							
Α					Е								Α					Ε							
В					Е								В					Е							
С					Е								С					Е							
D					Е								D					Е							
Ε					Е								Е					Е							
F					Ε								F					Ε							
			CON	NTRO	)L C	OE	)E "	7''					Α	CTIV	ITY (	COD	E (C	ON.	TRO	OL (	COI	DE '	"7"	)	

			CON	ITRO	)L C	OE	)E '	<b>'7''</b>				
	HR	MIN	ACT		ON	S	М	Т	W	Т	F	S
	(HH)	(MM)			0	1	2	3	4	5	6	7
0				Е								
1				Е								
2				ш								
3				Е								
4				Е								
5				Е								
6				Е								
7				Е								
8				Е								
9				Е								
Α				Е								
В				Е								
С				Е								
D				Е								
Ε				E								
F				Е								

- 1 = TYPE OF SIMULTANEOUS PHASE TERMINATION
- 2 = MAX 2
- 3 = MAX 3
- 4 = CONDITIONAL SERVICE (1ST SELECT)
- 5 = CONDITIONAL SERVICE (2ND SELECT)
- 6 = ENERGIZE AUXILIARY OUTPUT
- 7 = ENERGIZE AUXILIARY OUTPUT
- 8 = ENERGIZE AUXILIARY OUTPUT
- 9 = MAX RECALL 1
- A = TRAFFIC ACTUATED MAX 2 OPERATION
- B = MAX RECALL2
- C = YELLOW YIELD COORDINATION
- D = YELLOW YIELD COORDINATION
- E = FREE OPERATION
- F = FLASHING OPERATION

DATE START:	6/18/98
DATE SUPERSEDED:	

LOCATION: S/B RTE. 405 FWY @ AVALON

\* = Default Settings

			CONTRO	L CODE "	D"				
	SYSTEM DETECTOR	INPUT	DELAY	CARRYOVER		INPUT	DELAY	CARRYOVER	
	0	SLOT	1	3		SLOT	2	4	
0		l1				J1			0
1		I2U				J2U			1
2		I2L				J2L			2
3		I3U				J3U			3
4		I3L				J3L			4
5		14				J4			5
6		<b>I</b> 5				J5			6
7		16U				J6U			7
8		I6L				J6L			8
9		I7U				J7U			9
Α		I7L				J7L			Α
В		<b>I</b> 8				J8			В
С		I9U				J9U			С
D		I9L				J9L			D

X = New Settings

NO	OTES AND	REMARKS:
DATE START:	6/18/98	DETECTOR COUNT
DATE SUPERSEDED:		SAMPLING PERIOD
		F-C-F = 120
		E-1-6

RTE: 405

P.M.: 11.22 5 OF 6

CO: L.A.

																CC	TNC	RO	L C	ODE "	Ε"					(TH	IIS F	PAG	E @	FC	F=12	23)				
	INPUT		<b>C</b> (	PHA	SE [	DISP	LAY)	)		D	(FUI	NCTI	ON I	DISP	LAY	)				INPUT		E (	PHA	SE D	ISPI	AY)			F	(FL	JNCTI	ON	DISP	LAY)	,	
	FILE									RL	YL			EX	СО	CA	Т3			FILE									RL	YL			EX	СО	CA	Т3
		1	2	3	4	5	6	7	8	1	2	3	4	5	6	7	8				1	2	3	4	5	6	7	8	1	2	3	4	5	6	7	8
		*												*	*					J1					*								*	*		
1	I2U		*											*	*					J2U						*							*	*		
2	I2L		*											*	*				2	J2L						*							*	*		
	I3U		*											*	*				3	J3U						*							*	*		
4	I3L		*											*					4	J3L						*							*			
	14		*													*	*		5	J4						*									*	*
6	<b>I</b> 5			*										*	*				6	J5							*						*	*		
7	I6U				*									*	*				7	J6U								*					*	*		
8	I6L				*									*	*				8	J6L								*					*	*		
9	I7U				*									*	*				9	J7U								*					*	*		
Α	I7L				*									*					Α	J7L								*					*			
В	<b>I</b> 8				*											*	*		В	J8								*							*	*
С	I9U	*												*	*				С	J9U					*								*	*		
D	I9L			*										*	*				D	J9L							*						*	*		

NOTE: Default settings (\*) should remain unless replaced with new settings (X).

LC	CATIO	N:	S/B RTE.	405	FWY	@ <b>A</b>	VAL	ON									С	0:	L.A.	!	RTE	: 405	5	P.M.: 11.22	6 OF 6
CC	ONTROI	L CODE	E ''8'':				HOL	IDAY	1-16	(C-0-	8=11)	С	ONTRO	L CODI	E "8":				HOLII	DAY 1	17-32	2 (C-0	-8=12)	NOTES:	
	DAY	YEAR	MONTH					TY					DAY	YEAR	MONTH					TY					
	(DD)	(YY)	(M)		1	2	3	4	5	6	7		(DD)	(YY)	(M)		1	2	3	4	5	6	7	.	
0				Е								0				ш									
1				Е								1				ш								.	
2				шш								2				шш									
3				E								3				E									
5				E								5				E									
6				E			1		1	1		6				E		1		1				•	
7				E								7				E								•	
8				E								8				E								•	
9				Е								9				Е								•	
Α				Е								Α				Е								•	
В				Ε								В				Е								•	
С				Е								С				Е									
D				Е								D				Е									
Е				Ε								E				Е									
F				Ε								F				Е									
							-	-		-			-	•										•	
С	ONTRO	L COD	E "9":		Н	IOLID	AY E			(C-0-	9=11)	С	ONTRO	L CODI	E "9":		Н	OLIDA	Y EV			2(C-0	-9=12)		
С	TIME	L COD	E "9": OFS		Н			TY	PE		_	С	TIME	L CODI	E "9":		Н			ΤY	PΕ	,	,	:	
					H	OLID,	AY E			(C-0-	9=11)				1		H(	OLID/	Y EV			2(C-0	,	:   ·   ·	
0	TIME			E				TY	PE		_	0	TIME		1	E				ΤY	PΕ	,	,	:	
0	TIME			Ε				TY	PE		_	0	TIME		1	Ε				ΤY	PΕ	,	,		
0 1 2	TIME			E				TY	PE		_	0 1 2	TIME		1	E				ΤY	PΕ	,	,		
0	TIME			E				TY	PE		_	0	TIME		1	ЕЕ				ΤY	PΕ	,	,		
0 1 2 3 4	TIME			шшшш				TY	PE		_	0 1 2 3 4	TIME		1	шшшш				ΤY	PΕ	,	,		
0 1 2 3 4 5	TIME			шшшшш				TY	PE		_	0 1 2 3 4 5	TIME		1	шшшш				ΤY	PΕ	,	,		
0 1 2 3 4 5 6	TIME			шшшшшш				TY	PE		_	0 1 2 3 4 5	TIME		1	шшшшшш				ΤY	PΕ	,	,		
0 1 2 3 4 5	TIME							TY	PE		_	0 1 2 3 4 5	TIME		1					ΤY	PΕ	,	,		
0 1 2 3 4 5 6	TIME			шшшшшш				TY	PE		_	0 1 2 3 4 5	TIME (HHMM)		1	шшшшшш				ΤY	PΕ	,	,		
0 1 2 3 4 5 6 7 8	TIME							TY	PE		_	0 1 2 3 4 5 6 7 8	TIME (HHMM)		1					ΤY	PΕ	,	,		
0 1 2 3 4 5 6 7 8 9	TIME							TY	PE		_	0 1 2 3 4 5 6 7 8	TIME (HHMM)		1					ΤY	PΕ	,	,		
0 1 2 3 4 5 6 7 8 9 A B	TIME							TY	PE		_	0 1 2 3 4 5 6 7 8	TIME (HHMM)		1					ΤY	PΕ	,	,		
0 1 2 3 4 5 6 7 8 9 A B	TIME							TY	PE		_	0 1 2 3 4 5 6 7 8 9 A B	TIME (HHMM)		1					ΤY	PΕ	,	,	DATE STAR	rT:
0 1 2 3 4 5 6 7 8 9 A B C	TIME							TY	PE		_	0 1 2 3 4 5 6 7 8 9 A	TIME (HHMM)		1					ΤY	PΕ	,	,		
0 1 2 3 4 5 6 7 8 9 A B	TIME							TY	PE		_	0 1 2 3 4 5 6 7 8 9 A B	TIME (HHMM)		1					ΤY	PΕ	,	,	DATE STAR	8

**TSCP: 3.03** 

Location: R110 NB @ 220th St - Figueroa St

System: Master At: N/A District: 07

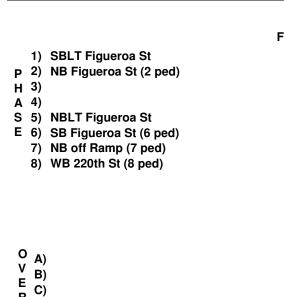
I/C: N/A

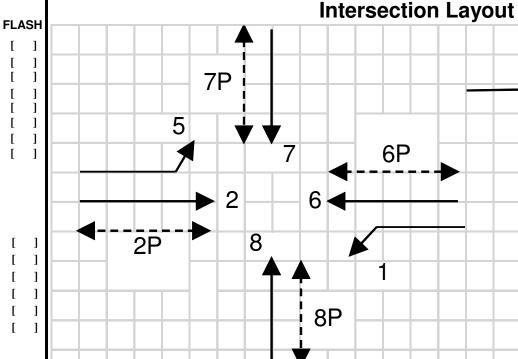
Designed By: HD Installed By: HD

Service Info:

 Timing Change:
 Date Start:
 Date End:
 Designed:
 Installed:

 9/23/2020
 9/10/2020
 9/23/2020





**Comments and Notes:** 

R D) L E) P F)

#### **RAM Checksum**

Page 2: D803	Page 8: 85AF
Page 3: EC24	Page 9: D2FD
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Post Mile: 6.76 Printed: 5/11/2021

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### Location: R110 NB @ 220th St - Figueroa St

	Cabinet (9-3)
	332
	Configuration
1	CALTRANS

**Vehicle Min** 

Vehicle Max

Pedestrian

Bicycle

Phase Recalls (2-1-1-2)

Phases ( 2-1-1-1 )		
Permitted	125678	
Restricted		

## **CONFIGURATION PHASE FLAGS**

Phase Locks (2-1-1-3)			
Red	15		
Yellow	. 2 6		
Force/Max			

Phase Features ( 2-1-1-4 )			
Double Entry			
Rest In Walk			
Rest In Red			
Walk 2			
Max Green 2			
Max Green 3			

Startup ( 2-1-1-5 )			
First Green Phases	.26		
Yellow Start Phases	8		
Vehicle Calls	125678		
Pedestrian Calls	.2678		
Yellow Start Overlaps			
Startup All-Red	6.0		

		_	
Ca	all To Phase ( 2-1-2-1 )		Omit On Green
1		1	
2		2	
3		3	
4		4	
5		5	
6		6	
7		7	
8		8	

Flashing Colors ( 2-1-2-	2)
Yellow Flash Phases	
Yellow Flash Overlaps	
Flash In Red Phases	
Flash In Red Overlaps	

Special Operation ( 2-1-2-3 )		
Single Exit Phase		
Driveway Signal Phases		
Driveway Signal Overlaps		
Leading Ped Phases		

Protected Permissive ( 2-1-2-4 )		
Protected Permissive		

Pedestrian ( 2-1-3 )		
P1		
P2	. 2	
Р3		
P4	7.	
P5		
P6	6	
P7		
P8	8	

Overlap ( 2-1-4 )				
Overlap	Parent	Omit	No Start	Not
Α				
В				
С				
D				
E				
F				

**Max 2 Extension** 

0

0

Max/Gap Out (2-7)

Max Cnt

Gap Cnt

Н
A
S
Ε
F
T
M
Ν
G

Overlap (2-4)

Green

Yellow

Red

Phase ( 2-2 )	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Walk 1	0	7	0	10	0	7	7	7
Flash Don't Walk	0	14	0	10	0	22	21	21
Minimum Green	10	10	10	10	10	10	10	10
Det Limit	0	0	10	10	0	0	0	0
Max Initial	0	20	10	10	0	20	0	0
Max Green 1	20	30	50	50	25	30	20	20
Max Green 2	20	30	50	50	25	30	20	20
Max Green 3	20	30	50	50	25	30	20	20
Extension	2.5	4.0	5.0	5.0	3.0	4.0	3.0	3.0
Maximum Gap	2.5	5.0	5.0	5.0	3.0	5.0	3.0	3.0
Minimum Gap	2.5	3.0	5.0	5.0	3.0	3.0	3.0	3.0
Add Per Vehicle	0.0	2.0	1.0	1.0	0.0	2.0	0.0	0.0
Reduce Gap By	0.0	0.1	0.0	0.0	0.0	0.1	0.0	0.0
Reduce Every	0.0	1.0	1.0	1.0	0.0	1.0	0.0	0.0
Yellow	3.7	4.4	5.0	5.0	3.7	4.4	4.1	4.1
All-Red	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Ped/Bike (2-3)	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Walk 2	0	0	0	0	0	0	0	0
Delay/Early Walk	0	0	0	0	0	0	0	0
Solid Don't Walk	0	0	0	0	0	0	0	0
Bike Green	0	0	0	0	0	0	0	0
Bike All-Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

#### **OVERLAP TIMING**

0.0

5.0

0.0

В

0.0

5.0

0.0

Α

0.0

5.0

0.0

#### **Red Revert** Red Revert (2-5) Е F Time 5.0 0.0 0.0 0.0 All-Red Sec/Min (2-6) 5.0 5.0 0.0 All-Red Sec/Min: OFF 0.0 0.0

Post Mile: 6.76 PAGE 3 CHECKSUM: EC24 Printed: 5/11/2021

Loc	Local Plan 19 (7-1) TIMING DATA							COORDINATION									Master Timer Sync (7-A)			
						Offset	s]	Gr	een Fa	actors o	r Pres	s [F] to	Select	Force-C	Off	Enable in Plans			Plans	
		Cycle	Multi	Lag Gap	Α	В	С	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-	ji 📗	1-9			
Plan 1	Green Factor															1	1-19			
Plan 2	Green Factor									<u> </u>						2	21-29			
Plan 3	Green Factor																Master Si	ub Mas	ster	
Plan 4	Green Factor															-[	Output			
Plan 5	Green Factor															F	REE PLA	N PHA	ASE FLAGS	
Plan 6	Green Factor															(7	7-E) Free	<del>)</del>		
Plan 7	Green Factor									<u> </u>							Lag . 2 . 4 . 6 .	. 8	Omit	
Plan 8	Green Factor									<del>                                     </del>							Veh Mir		Veh Max	
Plan 9	Green Factor									1						┟┝	. 2 6 . Ped		Bike	
																ᆘ				
Lo	cal Plan 1	9 (7	7-1) P	HASE FLA	\GS											il	Cond		Cond Grn	
	Lag	<u> </u>	Sync	Hold	10.0	Omit		Veh N	/lin	Veh N	lax	Ped	l k	Bike	е	łΕ			10	
Plan																<u>i</u> -,	 IAUUAN	L CO	 MMANDS	
Plan	2																nual Plan		Plan: 1-29	
Plan	3															F	Plan O	ffSet	254 = Flash 255 = Free	
Plan	4															Ł		A	Offset A, B, or C	
Plan	5																		verride (4-2)	
Plan	6															#				
Plan	7															2				
Plan	8																Detector R	Reset	(4-3)	
Plan	9															۱ ۱	Local Man	ual (4-4	) OFF	

## Local Plan 11...19 (7-2) TIMING DATA

### **COORDINATION**

[ Offsets ] Green Factors or Press [F] to Select Force-Off

		Cycle	Multi	Lag Gap	Α	В	С	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Plan 11	Green Factor														
Plan 12	Green Factor														
Plan 13	Green Factor														
Plan 14	Green Factor														
Plan 15	Green Factor														
Plan 16	Green Factor														
Plan 17	Green Factor														
Plan 18	Green Factor														
Plan 19	Green Factor														

## Local Plan 11...19 (7-2) PHASE FLAGS

	Lag	Sync	Hold	Omit	Veh Min	Veh Max	Ped	Bike
Plan 11								
Plan 12								
Plan 13								
Plan 14								
Plan 15								
Plan 16								
Plan 17								
Plan 18								
Plan 19								

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## Local Plan 21...29 (7-3) TIMING DATA

### **COORDINATION**

[ Offsets ]

Green Factors or Press [F] to Select Force-Off

					ι,	3113013	,	•	cii i uc			[. ] . · ·		0.00	
		Cycle	Multi	Lag Gap	Α	В	С	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-
Plan 21	Green Factor														
Plan 22	Green Factor														
Plan 23	Green Factor														
Plan 24	Green Factor														
Plan 25	Green Factor														
Plan 26	Green Factor														
Plan 27	Green Factor														
Plan 28	Green Factor														
Plan 29	Green Factor														
															Ш

## Local Plan 21...29 (7-3) PHASE FLAGS

	Lag	Sync	Hold	Omit	Veh Min	Veh Max	Ped	Bike
Plan 21								
Plan 22								
Plan 23								
Plan 24								
Plan 25								
Plan 26								
Plan 27								
Plan 28								
Plan 29								

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## **DETECTORS**

Dete	ector Attributes (5-1)			Slot	Detector Configuration (5-2)					
Det		Phases	Lock		Det	Delay	Extend		Port	
	COUNT+CALL+EXTEND	1	NO	I1U	1			10	3.2	
2	COUNT+CALL+EXTEND	1	NO	I1L	2			10	7.2	
3	COUNT+CALL+EXTEND	. 2	NO	I2U	3			10	1.1	
4	COUNT+CALL+EXTEND	. 2	NO	I2L	4			10	1.5	
5	COUNT+CALL+EXTEND	. 2	NO	I3U	5			10	4.5	
6	CALL+EXTEND	. 2	NO	I3L	6			10	6.2	
	LIMITED	. 2	NO	I4U	7			10	2.1	
8	COUNT+CALL+EXTEND	. 2	NO	I4L	8			10	7.4	
9	COUNT+CALL+EXTEND	3	NO	I5U	9			10	3.4	
10	COUNT+CALL+EXTEND	3	NO	I5L	10			10	7.6	
11	COUNT+CALL+EXTEND	4	NO	I6U	11			10	1.3	
12	COUNT+CALL+EXTEND	4	NO	I6L	12			10	1.7	
13	COUNT+CALL+EXTEND	4	NO	I7U	13			10	4.7	
14	CALL+EXTEND	4	NO	I7L	14			10	6.4	
15	LIMITED	4	NO	I8U	15			10	2.3	
16	COUNT+CALL+EXTEND	4	NO	I8L	16			10	7.8	
17	COUNT+CALL+EXTEND	1	NO	I9U	17			10	3.6	
18	COUNT+CALL+EXTEND	3	NO	I9L	18			10	3.8	
19	COUNT+CALL+EXTEND	. 2	NO	I10U	19			10	4.1	
20	COUNT+CALL+EXTEND	4	NO	I10L	20			10	4.2	
21	COUNT+CALL+EXTEND	5	NO	J1U	21			10	3.1	
	COUNT+CALL+EXTEND	5	NO	J1L	22			10	7.1	
	COUNT+CALL+EXTEND	6	NO	J2U	23			10	1.2	
	COUNT+CALL+EXTEND	6	NO	J2L	24			10	1.6	
	COUNT+CALL+EXTEND	6	NO	J3U	25			10	4.6	
	CALL+EXTEND	6	NO	J3L	26			10	6.3	
	LIMITED	6	NO	J4U	27			10	2.2	
28	COUNT+CALL+EXTEND	6	NO	J4L	28			10	7.3	
	COUNT+CALL+EXTEND	7.	NO	J5U	29			10	3.3	
	COUNT+CALL+EXTEND	7.	NO	J5L	30			10	7.5	
	COUNT+CALL+EXTEND	8	NO	J6U	31			10	1.4	
	COUNT+CALL+EXTEND	8	NO	J6L	32			10	1.8	
	COUNT+CALL+EXTEND	8	NO	J7U	33			10	4.8	
	CALL+EXTEND	8	NO	J7L	34			10	6.5	
	LIMITED	8	NO	J8U	35			10	2.4	
	COUNT+CALL+EXTEND	8	NO	J8L	36			10	7.7	
	COUNT+CALL+EXTEND	5	NO	J9U	37			10	3.5	
	COUNT+CALL+EXTEND	7.	NO	J9L	38			10	3.7	
	COUNT+CALL+EXTEND	6	NO	J10U	39			10	4.3	
	COUNT+CALL+EXTEND	8	NO	J10L	40			10	4.4	
	PEDESTRIAN	.2	NO	I12U	41			10	5.1	
	PEDESTRIAN	7.	NO	112L	42			10	5.3	
	PEDESTRIAN	6	NO	I13U	43			10	5.2	
	PEDESTRIAN	8	NO	113L	44			10	5.4	
	-			IIUL				-		

Failure Times(5-3)	Minutes
<b>Maximum On Time</b>	
Fail Reset Time	

Failure Override (5-4)					
Detectors 1-8					
Detectors 9-16					
Detectors17-24					
Detectors 25-32					
Detectors 33-40					
Detectors 41-44					

System Detector Assignment (5-5)								
Sys Det	1	2	3	4	5	6	7	8
Det Nu								
Sys Det	9	10	11	12	13	14	15	16
Det Nu								

CIC Operation (5-6-1	)
Enable in Plans	

CIC Values (5-6-2)	Volume	Occupancy	Demand
Smoothing	0.66	0.66	0.66
Multiplier	4.0	0.33	
Exponent	0.50	1.00	

	Detec	Detector-to-Phase Assignment (5-6-3)							
Sys Det	1	2	3	4	5	6	7	8	
Phase									
Sys Det	9	10	11	12	13	14	15	16	
Phase									

## **Input File Port-Bit Assignments**

332 Cabinet - For Reference Only

	1	2	3	4	5	6	7	8	9	10	11	12	13	14
I-	3.2	1.1	4.5	2.1	3.4	1.3	4.7	2.3	3.6	4.1	6.6	5.1	5.2	<b>6.7</b>
	7.2	1.5	6.2	7.4	7.6	1.7	6.4	7.8	3.8	4.2	2.7	5.3	5.4	6.8
J-	3.1	1.2	4.6	2.2	3.3	1.4	4.8	2.4	3.5	4.3	2.8	5.5	5.6	2.5
	7.1	1.6	6.3	7.3	7.5	1.8	6.5	7.7	3.7	4.4	6.1	5.7	5.8	2.6

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### **TOD SCHEDULE**

Table 1	(8-2-1)		Table 2	2 (8-2-2)		Table 3	(8-2-3)			Table 4	(8-2-4)		Table 5	(8-2-5)		Table 6	(8-2-6)	
Time	Plan	os	Time	Plan	os	Time	Plan	os		Time		os	Time	Plan	os	Time	Plan	os
		Α			Α			Α	•			Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α	-			Α			Α			Α
		Α			Α			Α	-			Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α	-			Α			Α			Α
		Α			Α			Α	-			Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α				Α			Α			Α
		Α			Α			Α		_		Α			Α			Α
		Α			Α			Α				Α			Α			Α

### **WEEKDAY ASSIGNMENT**

	Weekday Table Assignments (8-2-7)									
,	Mon	Tue	Wed	Thu	Fri	Sat	Sun			
	1	1	1	1	1	2	2			

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#### **HOLIDAY TABLES**

Flo	Floating Holiday Table (8-2-8)									
#	Mnth	Week	DOW	Table						
1										
2										
3										
4										
5										
6										
7										
8										
9										
10										
11										
12										
13										
14										
15										
16										

_			le (8-2-9)	Table
#	Mnth	Day	DOW	Table
1				
2				
3				
4				
5				
6				
7				
8				
9				
10				
11				
12				
13				
14				
15				
16				

Daylight Saving (8-1)							
Enabled	YES	Month	Sunday				
	Start		2nd				
End NOV 1st							

Solar Clock Data (8-4)					
North Latitude	34				
West Longitude	118				
Local Time Zone	8				

Sabbatical Clock (8-5)						
Hebrew	Ped Recall					
Sabbath						
Holiday						

#### **TOD FUNCTIONS**

TO	TOD Functions (8-3)								
#	Start	End	DOW	Action	Phases				
1			• • • • • •						
2			• • • • • •						
3			• • • • • •						
4									
5									
6			• • • • • •						
7									
8									
9									
10			• • • • • •						
11			• • • • • •						
12			• • • • • •						
13									
14									
15									
16			• • • • • •						

**Action Codes:** 

- 0. None
- 1. Permitted
- 2. Restricted
- 4. Veh Min Recall
- 5. Veh Max Recall
- 6. Ped Recall
- 7. Bike Recall
- 8. Red Lock
- 9. Yellow Lock
- 10. Force/Max Lock
- 11.Double Entry
- 12. Y-Coord C
- 13. Y-Coord D
- 14. Free
- 15. Flashing
- 16. Walk 2
- 17. Max Green 2

- 18. Max Green 3
- 19. Rest in Walk
- 20. Rest in Red
- 21. Free Lag Phases
- 22. Special Functions
- 23. Truck Preempt
- 24. Conditional Service
- 25. Conditional Service
- 26. Leading Ped
- 27. Traffic Actuated Max 2
- 41. Protected Permissive
- 42. Protected Permissive

Action Code = Phases added to normal setting

100+Action Code = Phases removed

200+Action Code = Phases replaced

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#### **COMMUNICATIONS**

	C2 (6-1-1)	C20 (6-1-2)	C21 (6-1-3)
Address			
Baud	1200	1200	1200
Protocol	AB3418	AB3418	AB3418
Data Bits	8	8	8
Parity	NONE	NONE	NONE
Stop Bits	1	1	1
RTS On Time	20	20	20
RTS Off Time	20	20	20
Handshaking	NORMAL	NORMAL	NORMAL
Access Level	0	0	0

### **SOFT LOGIC**

	ft Logic (						
#	Data	OP	Data	OP	Data	OP	Data
1							
2							
3							
4							
5							
6							
7							
8							
9							
10							
11							
12							
13							
14			·		·		
15							
16							

<sup>\*</sup>Refer to User's Manual for Data and OP Codes

### **CALLBACK NUMBERS**

Callback Numbers (6-33)								
Line Out								
Long Distance								
Local Toll								
Delay	10	10	10					
Area Code								
Phone Number								

### **NETWORK**

Network Parameters (6-4)						
Address						
Protocol	AB3418					
Port	27000					
Туре	STATIC					
<b>Central Access</b>						
Field Access						
ATSPM	OFF					

IP Address	0	0	0	0
Netmask	255	255	255	0
Broadcast	0	0	0	255
Gateway	0	0	0	254
Gateway				

#### **Access Levels:**

**0-Full Access** 

1-Status Only

2-Status, Set Pattern, Time

3-Status, Set Pattern, Time, Manual Plan

4-Reserved

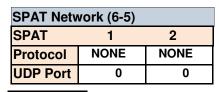
5-Full Access with No Set Pattern

6-Full Access with No Set Time

7-Full Access with No Set Pattern, Manual Plan

Plan

8-Full Access with No Set Time, Pattern, Manual Plan



IP Address 0 . 0 . 0 .

### **RAILROAD PREEMPTION**

RR 1	Timing	( 3-1-1 )
	Clear 1	15
	Clear 2	5
	Clear 3	
	Hold	
	Min Gr	
	Delay	
	Exit	
	Ped Clr	

Pi	nase Flags (3-1-	-2)	Pedestrian Flags (3-1-3)			Overlap Flags (3-1-4)			
Grn Hold	Yel Flash	Red Flash	Walk	Flash DW	Solid DW	Grn Hold	Yel Flash Red Flash		
. 2 5					.2.4.6.8				
. 2 5					.2.4.6.8				
1478				48	.26				

	Exit Pa	ramete	rs (3-1-5)			_
ł	Phase	Green	Ovrlap Green	Veh Permit/Call	Ped Permit/Call	
ł	. 2	.6		12345678	.2.4.6.8	
J	-					

(	Configuration (3-1-6)											
	PR	XR	Gate	Isld	APP		Sign	Sign		Max On	Latchin	
1										5	NO	
2			Valid Inp	uts: 1.	x, 2.x, 3.	x,	4.x, 5.x,	6.x, 7.x	Χ,	8.x x=1	to 8	
			Valid Out	puts: 11.	x <u>, 1</u> 2.x, <u>13</u> .	х,	14.x, 15.x,	16.x, 17.	х,	18.x x=1	to 8	

RR 2	Timing	( 3-2-1 )
_	Clear 1	15
	Clear 2	5
	Clear 3	
	Hold	
	Min Gr	
	Delay	
	Exit	
	Ped Clr	

Pl	hase Flags (3-2	-2)	Pede	estrian Flags (3	-2-3)	Overlap Flags (3-2-4)			
Grn Hold	Grn Hold Yel Flash Red Flash		Walk	Walk Flash DW Solid DW		Grn Hold Yel Flash		Red Flash	
47.					.2.4.6.8				
47.					.2.4.6.8				
1236				. 2 6	48				

1

Exit Paramet	ers (3-2-5)		
Phase Green	Ovrlap Green	Veh Permit	Ped Permit
.26		12345678	.2.4.6.8

Configuration (3-2-6)											
PR	XR	Gate	Isld	APP		Sign	Sign	Max On	Latchinç		
								5	NO		

✓ Valid Inputs: 1.x, 2.x, 3.x, 4.x, 5.x, 6.x, 7.x, 8.x x=1 to 8 Valid Outputs: 11.x, 12.x, 13.x, 14.x, 15.x, 16.x, 17.x, 18.x x=1 to 8

### **EMERGENCY VEHICLE PREEMPTION**

EVA	Preempt Timers		Phase Green	Overlap	
(3-A)	Delay	Clear	Max		Green
		30	30	.25	• • • • • •

Port	Latching	Phase Termination
	NO	ADVANCE

EVC	Preempt Timers		Phase Green	Overlap	
( <b>3-C</b> )	Delay	Clear	Max		Green
		30	30	16	• • • • •

Port	Latching	Phase Termination
	NO	ADVANCE

EVB	Preempt Timers		Phase Green	Overlap	
(3-B)	Delay	Clear	Max		Green
		30	30	47.	

Port	Latching	Phase Termination
	NO	ADVANCE

EVD (3-D)		Pre	empt Tim	ers	Phase Green	Overlap
	(3-D)	Delay	Clear	Max		Green
			30	30	38	• • • • •

Port	Latching	Phase Termination
	NO	ADVANCE

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INPU	TS	7 Wire I/C(2-1-5-1)			
510		Input	Port	Input	Port
Enable	NO	RR1		Free	
Max ON		RR2		D2	
Max OFF		RR3		D3	

Manual Control ( 2-1-5-2 )				
Input	Port			
Manual Advance				
Advance Enable				

Cabinet Status ( 2-1-5-3				
Input	Port			
Flash Bus				
Door Ajar				
Flash Sense	6.7			
Stop Time	6.8			

Special Function (2-1-5-4)					
Input	Poi	rt			
1					
2					
3					
4					

Battery Backup (2-1-5-5)				
Port	Operation			
	NORMAL			

Y-Coordination ( 2-1-5-6					
Port C Port D					

### **OUTPUTS**

Loadsw	itch Assi	gnments	s ( 2-1-6 )				
Α	1	2	22	3	4	24	9
В	5	6	26	7	8	28	10
Х	13	14	0	11	12	0	0

**Loadswitch Codes:** 

0 Unused (no output)

1-8 Vehicle 1-8

9-14 Overlap A-F

21-28 Ped 1-8

41-47 Special Functions

41 Protected Permissive Flashing Phase 1

43 Protected Permissive Flashing Phase 3

45 Protected Permissive Flashing Phase 5

47 Protected Permissive Flashing Phase 7

51-57 Special Functions

71-72 Seven Wire I/C

+ middle output of loadswitches 3 and 6 Channel 9 and 10

### **INTERVAL CONTROL**

Interval	Control
(3-3-1)	Time
Step 1	
Step 2	
Step 3	
Step 4	
Step 5	
Step 6	
Step 7	
Step 8	

Ph	Phase Control (3-3-2)					
Hold	Force	Advance				
<u> </u>						

Phase Recall (3-3-3)				
Veh Call	Ped Call	Int Call		

1 116	I hase I crimited (3-3-4)				
Phs Permit	Ped Permit	<b>Ovrlap Permit</b>			

Phase Permitted (3-3-4)

Configuration (3-3-5)					
Input	Port	Delay	<b>HRI Cross</b>		
1					
2					

HRI

	HRI Configuration (3-4)				
RailRoad	51		WAYSIDE	ATC	
Line		Subnode			
Group		Device			

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### TRANSIT PRIORITY

Local Pla	ans (3-E) 19 1119	Early	Green	Inhibit	Phase 1	Phase 2	Phase 3	Phase 4	Phase 5	Phase 6	Phase 7	Phase 8
		Green	Extend	Cycles	Minimum							
Plan 1	Green Factor		,									
Plan 2	Green Factor											
Plan 3	Green Factor											
Plan 4	Green Factor											
Plan 5	Green Factor											
Plan 6	Green Factor											
Plan 7	Green Factor											
Plan 8	Green Factor											
Plan 9	Green Factor											
Plan 11	Green Factor											
Plan 12	Green Factor											
Plan 13	Green Factor											
Plan 14	Green Factor											
Plan 15	Green Factor											
Plan 16	Green Factor											
Plan 17	Green Factor											
Plan 18	Green Factor											
Plan 19	Green Factor											

<b>Transit Prio</b>		Indicato	r Output		
Enable in Plans		Input	Type	Stop	Go
Plan 1-9		0.0	NONE	0	0
Plan 11-19		0.0	NONE	0	0

Queue Jump (3-E-B)			
Grn Hold	Hold Phase		

Free Plans (3-E	:-E)
Max Grn Hold	Hold Phase

Access Utiliti	ies (9-5)
Password	***
Timeout	30

### YELLOW YIELD COORDINATION

	_	_		į			_	Force	-Offs		_	_		_	_	_
Y-Coord Plans (7-C,D)	Long Grn	No Grn	Offset	Perm	-1-	-2-	-3-	-4-	-5-	-6-	-7-	-8-	Coord	Lag	Min Recall	Restricted
Plan C													.26	.2.4.6.8		
Plan D													.26	.2.4.6.8		

### TRUCK PRIORITY

Truck Priority (3-F)	Passage	CarryOver	Clearance	Next Priority	Phase Green						Slave Output
						0.0	0.0	0.0	0	0.0	0

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APPENDIX: HCM Queuing Analysis

	•	<b>←</b>	•	•	<b>†</b>	ļ	4
Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	29	29	514	407	1207	792	251
v/c Ratio	0.14	0.14	0.32	0.67	0.40	0.27	0.24
Control Delay	33.0	33.0	0.5	35.9	3.3	10.3	2.5
Queue Delay	0.0	0.0	0.0	0.0	0.3	0.0	0.0
Total Delay	33.0	33.0	0.5	35.9	3.5	10.3	2.5
Queue Length 50th (ft)	13	13	0	98	97	79	0
Queue Length 95th (ft)	38	38	0	135	127	117	37
Internal Link Dist (ft)		1090			308	844	
Turn Bay Length (ft)	360		360	225			265
Base Capacity (vph)	481	484	1583	914	2995	2960	1026
Starvation Cap Reductn	0	0	0	0	979	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.06	0.06	0.32	0.45	0.60	0.27	0.24
Intersection Summary							

	۶	<b>→</b>	$\rightarrow$	<b>†</b>	ļ	4
Lane Group	EBL	EBT	EBR	NBT	SBT	SBR
Lane Group Flow (vph)	423	4	523	1293	635	204
v/c Ratio	0.31	0.00	0.73	0.84	0.41	0.25
Control Delay	13.7	11.2	18.2	22.2	13.1	2.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	13.7	11.2	18.2	22.2	13.1	2.9
Queue Length 50th (ft)	55	0	113	210	80	0
Queue Length 95th (ft)	82	3	218	#345	122	32
Internal Link Dist (ft)		1457		794	308	
Turn Bay Length (ft)	195		145			
Base Capacity (vph)	1384	1427	730	1540	1546	806
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.00	0.72	0.84	0.41	0.25
Intersection Summary						

intersection Summary

Queue shown is maximum after two cycles.

<sup># 95</sup>th percentile volume exceeds capacity, queue may be longer.

Lane Group         EBT         EBR         WBT         WBR         NBL         NBT         SBL         SBT           Lane Group Flow (vph)         264         78         337         150         513         798         192         747           v/c Ratio         4.98         0.19         1.05         0.36         1.50         0.84         0.77         0.88           Control Delay         1861.9         4.9         112.5         18.5         277.5         52.8         74.4         49.2           Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0         0.0           Total Delay         1861.9         4.9         112.5         18.5         277.5         52.8         74.4         49.2
v/c Ratio     4.98     0.19     1.05     0.36     1.50     0.84     0.77     0.88       Control Delay     1861.9     4.9     112.5     18.5     277.5     52.8     74.4     49.2       Queue Delay     0.0     0.0     0.0     0.0     0.0     0.0     0.0     0.0
Control Delay         1861.9         4.9         112.5         18.5         277.5         52.8         74.4         49.2           Queue Delay         0.0         0
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0
•
Total Delay 1861.9 4.9 112.5 18.5 277.5 52.8 74.4 49.2
Queue Length 50th (ft) ~386 0 ~316 34 ~609 326 159 250
Queue Length 95th (ft) #566 26 #508 97 #829 #458 238 330
Internal Link Dist (ft) 1163 595 462 664
Turn Bay Length (ft) 120 290 105
Base Capacity (vph) 53 421 321 421 341 946 307 896
Starvation Cap Reductn 0 0 0 0 0 0
Spillback Cap Reductn 0 0 0 0 0 0
Storage Cap Reductn 0 0 0 0 0 0
Reduced v/c Ratio 4.98 0.19 1.05 0.36 1.50 0.84 0.63 0.83

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	•	<b>←</b>	•	•	<b>†</b>	ļ	4
Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	52	51	486	275	1077	1552	567
v/c Ratio	0.24	0.23	0.31	0.55	0.38	0.53	0.49
Control Delay	34.1	34.0	0.5	35.8	3.9	12.6	2.8
Queue Delay	0.0	0.0	0.0	0.0	0.3	0.0	0.0
Total Delay	34.1	34.0	0.5	35.8	4.3	12.6	2.8
Queue Length 50th (ft)	25	24	0	67	82	172	0
Queue Length 95th (ft)	57	56	0	100	121	248	50
Internal Link Dist (ft)		1090			310	844	
Turn Bay Length (ft)	360		360	225			265
Base Capacity (vph)	481	483	1583	914	2798	2912	1148
Starvation Cap Reductn	0	0	0	0	1023	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.11	0.31	0.30	0.61	0.53	0.49
Intersection Summary							

	۶	<b>→</b>	•	<b>†</b>	ļ	4
Lane Group	EBL	EBT	EBR	NBT	SBT	SBR
Lane Group Flow (vph)	234	245	379	1442	1133	462
v/c Ratio	0.17	0.18	0.59	0.93	0.73	0.48
Control Delay	12.7	12.7	17.6	29.4	17.8	3.3
Queue Delay	0.0	0.0	0.0	0.0	0.4	0.0
Total Delay	12.7	12.7	17.6	29.4	18.1	3.3
Queue Length 50th (ft)	28	31	96	246	173	0
Queue Length 95th (ft)	47	50	171	#410	250	46
Internal Link Dist (ft)		1457		794	310	
Turn Bay Length (ft)	195		145			
Base Capacity (vph)	1384	1427	663	1545	1558	955
Starvation Cap Reductn	0	0	0	0	103	2
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.17	0.17	0.57	0.93	0.78	0.48
Intersection Summary						

<sup># 95</sup>th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	<b>→</b>	•	<b>—</b>	•	4	<b>†</b>	-	<b>↓</b>
Lane Group	EBT	EBR	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	252	91	184	76	463	591	92	1240
v/c Ratio	5.14	0.21	0.76	0.22	1.29	0.49	0.53	1.44
Control Delay	1929.6	7.6	69.5	5.3	190.1	33.7	65.1	239.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	1929.6	7.6	69.5	5.3	190.1	33.7	65.1	239.9
Queue Length 50th (ft)	~347	0	142	0	~470	186	72	~696
Queue Length 95th (ft)	#551	39	224	24	#733	282	131	#906
Internal Link Dist (ft)	1163		595			462		664
Turn Bay Length (ft)				120	290		105	
Base Capacity (vph)	49	437	341	437	359	1217	323	859
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	5.14	0.21	0.54	0.17	1.29	0.49	0.28	1.44

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	•	<b>←</b>	•	•	<b>†</b>	ļ	4
Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	37	38	514	560	1225	801	251
v/c Ratio	0.17	0.18	0.32	0.74	0.41	0.29	0.26
Control Delay	33.4	33.4	0.5	35.0	3.4	12.5	3.0
Queue Delay	0.0	0.0	0.0	0.0	0.3	0.0	0.0
Total Delay	33.4	33.4	0.5	35.0	3.7	12.5	3.0
Queue Length 50th (ft)	17	17	0	135	98	89	0
Queue Length 95th (ft)	46	46	0	175	135	133	42
Internal Link Dist (ft)		1090			308	844	
Turn Bay Length (ft)	360		360	225			265
Base Capacity (vph)	481	484	1583	921	2989	2735	967
Starvation Cap Reductn	0	0	0	0	966	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.08	0.32	0.61	0.61	0.29	0.26
Intersection Summary							

	•	<b>→</b>	$\rightarrow$	<b>†</b>	ļ	4
Lane Group	EBL	EBT	EBR	NBT	SBT	SBR
Lane Group Flow (vph)	423	4	600	1500	660	204
v/c Ratio	0.31	0.00	0.84	0.98	0.43	0.25
Control Delay	13.6	11.0	26.0	38.9	13.4	2.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	13.6	11.0	26.0	38.9	13.4	2.9
Queue Length 50th (ft)	53	0	149	278	87	0
Queue Length 95th (ft)	82	3	#332	#441	127	32
Internal Link Dist (ft)		1457		794	308	
Turn Bay Length (ft)	195		145			
Base Capacity (vph)	1384	1427	723	1525	1535	802
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.31	0.00	0.83	0.98	0.43	0.25
Intersection Summary						

<sup>95</sup>th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Lane Group         EBT         EBR         WBT         WBR         NBL         NBT         SBL         SBT           Lane Group Flow (vph)         268         78         337         150         513         798         192         747           v/c Ratio         5.15         0.19         1.05         0.36         1.50         0.84         0.77         0.88           Control Delay         1931.6         4.9         113.2         18.5         277.5         52.8         74.4         49.2           Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0
v/c Ratio       5.15       0.19       1.05       0.36       1.50       0.84       0.77       0.88         Control Delay       1931.6       4.9       113.2       18.5       277.5       52.8       74.4       49.2
Control Delay 1931.6 4.9 113.2 18.5 277.5 52.8 74.4 49.2
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
Total Delay 1931.6 4.9 113.2 18.5 277.5 52.8 74.4 49.2
Queue Length 50th (ft) ~394 0 ~316 34 ~609 326 159 250
Queue Length 95th (ft) #574 26 #509 97 #829 #458 238 330
Internal Link Dist (ft) 1163 595 462 664
Turn Bay Length (ft) 120 290 105
Base Capacity (vph) 52 421 321 421 341 946 307 896
Starvation Cap Reductn 0 0 0 0 0 0 0
Spillback Cap Reductn 0 0 0 0 0 0 0
Storage Cap Reductn 0 0 0 0 0 0 0
Reduced v/c Ratio 5.15 0.19 1.05 0.36 1.50 0.84 0.63 0.83

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	•	<b>←</b>	•	4	<b>†</b>	ļ	4
Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	70	69	486	370	1088	1570	567
v/c Ratio	0.31	0.30	0.31	0.64	0.39	0.57	0.51
Control Delay	34.8	34.6	0.5	35.9	4.2	14.7	3.1
Queue Delay	0.0	0.0	0.0	0.0	0.4	0.0	0.0
Total Delay	34.8	34.6	0.5	35.9	4.6	14.7	3.1
Queue Length 50th (ft)	33	33	0	90	83	188	0
Queue Length 95th (ft)	71	70	0	125	132	280	56
Internal Link Dist (ft)		1090			308	844	
Turn Bay Length (ft)	360		360	225			265
Base Capacity (vph)	481	483	1583	914	2778	2766	1119
Starvation Cap Reductn	0	0	0	0	1006	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.14	0.31	0.40	0.61	0.57	0.51
Intersection Summary							

	•	<b>→</b>	•	<b>†</b>	ļ	4
Lane Group	EBL	EBT	EBR	NBT	SBT	SBR
Lane Group Flow (vph)	234	245	546	1571	1186	462
v/c Ratio	0.17	0.17	0.84	1.03	0.77	0.49
Control Delay	12.4	12.4	29.5	51.4	19.4	3.3
Queue Delay	0.0	0.0	0.0	0.0	0.4	0.0
Total Delay	12.4	12.4	29.5	51.4	19.7	3.3
Queue Length 50th (ft)	27	30	163	~342	192	0
Queue Length 95th (ft)	47	50	#333	#469	267	46
Internal Link Dist (ft)		1457		794	308	
Turn Bay Length (ft)	195		145			
Base Capacity (vph)	1384	1427	663	1523	1535	948
Starvation Cap Reductn	0	0	0	0	75	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.17	0.17	0.82	1.03	0.81	0.49

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Control Delay       2024.5       7.6       69.7       5.3       190.1       33.7       65.1       240.0         Queue Delay       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0         Total Delay       2024.5       7.6       69.7       5.3       190.1       33.7       65.1       240.0         Queue Length 50th (ft)       ~357       0       142       0       ~470       186       72       ~696         Queue Length 95th (ft)       #562       39       224       24       #733       282       131       #906         Internal Link Dist (ft)       1163       595       462       664         Turn Bay Length (ft)       120       290       105
v/c Ratio       5.38       0.21       0.76       0.22       1.29       0.49       0.53       1.44         Control Delay       2024.5       7.6       69.7       5.3       190.1       33.7       65.1       240.0         Queue Delay       0.0       0.0       0.0       0.0       0.0       0.0       0.0         Total Delay       2024.5       7.6       69.7       5.3       190.1       33.7       65.1       240.0         Queue Length 50th (ft)       ~357       0       142       0       ~470       186       72       ~696         Queue Length 95th (ft)       #562       39       224       24       #733       282       131       #906         Internal Link Dist (ft)       1163       595       462       664         Turn Bay Length (ft)       120       290       105
Control Delay       2024.5       7.6       69.7       5.3       190.1       33.7       65.1       240.0         Queue Delay       0.0       0.0       0.0       0.0       0.0       0.0       0.0       0.0         Total Delay       2024.5       7.6       69.7       5.3       190.1       33.7       65.1       240.0         Queue Length 50th (ft)       ~357       0       142       0       ~470       186       72       ~696         Queue Length 95th (ft)       #562       39       224       24       #733       282       131       #906         Internal Link Dist (ft)       1163       595       462       664         Turn Bay Length (ft)       120       290       105
Queue Delay       0.0
Total Delay 2024.5 7.6 69.7 5.3 190.1 33.7 65.1 240.0 Queue Length 50th (ft) ~357 0 142 0 ~470 186 72 ~696 Queue Length 95th (ft) #562 39 224 24 #733 282 131 #906 Internal Link Dist (ft) 1163 595 462 664 Turn Bay Length (ft) 120 290 105
Queue Length 50th (ft)       ~357       0       142       0       ~470       186       72       ~696         Queue Length 95th (ft)       #562       39       224       24       #733       282       131       #906         Internal Link Dist (ft)       1163       595       462       664         Turn Bay Length (ft)       120       290       105
Queue Length 95th (ft)       #562       39       224       24       #733       282       131       #906         Internal Link Dist (ft)       1163       595       462       664         Turn Bay Length (ft)       120       290       105
Internal Link Dist (ft) 1163 595 462 664  Turn Bay Length (ft) 120 290 105
Turn Bay Length (ft) 120 290 105
0 " ( 1)
Base Capacity (vph) 48 437 340 437 359 1216 323 859
Starvation Cap Reductn 0 0 0 0 0 0 0
Spillback Cap Reductn 0 0 0 0 0 0
Storage Cap Reductn 0 0 0 0 0 0 0
Reduced v/c Ratio 5.38 0.21 0.54 0.17 1.29 0.49 0.28 1.44

Intersection Summary

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	•	<b>←</b>	•	4	<b>†</b>	ļ	4
Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	90	89	560	712	1316	1016	261
v/c Ratio	0.38	0.37	0.35	0.78	0.48	0.45	0.31
Control Delay	35.3	35.2	0.6	33.7	5.1	18.6	4.0
Queue Delay	0.0	0.0	0.0	0.0	0.5	0.0	0.0
Total Delay	35.3	35.2	0.6	33.7	5.6	18.6	4.0
Queue Length 50th (ft)	44	43	0	169	115	132	0
Queue Length 95th (ft)	85	84	0	210	188	205	51
Internal Link Dist (ft)		1090			308	844	
Turn Bay Length (ft)	360		360	200			265
Base Capacity (vph)	481	483	1583	971	2751	2241	843
Starvation Cap Reductn	0	0	0	0	884	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.19	0.18	0.35	0.73	0.70	0.45	0.31
Intersection Summary							

	-	•	•	•	1	<b>†</b>	-	<b>↓</b>
Lane Group	EBT	EBR	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	323	117	351	157	534	849	200	840
v/c Ratio	6.59	0.27	1.14	0.38	1.58	0.88	0.79	0.95
Control Delay	2567.8	9.0	140.4	19.7	311.9	56.2	75.9	58.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	2567.8	9.0	140.4	19.7	311.9	56.2	75.9	58.4
Queue Length 50th (ft)	~497	0	~348	39	~647	357	165	302
Queue Length 95th (ft)	#689	51	#543	105	#869	#510	248	#432
Internal Link Dist (ft)	1163		595			462		664
Turn Bay Length (ft)				120	290		105	
Base Capacity (vph)	49	429	308	417	337	962	303	889
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	6.59	0.27	1.14	0.38	1.58	0.88	0.66	0.94

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

## 34: Lenardo Dr & I-405 SB Ramps

	-	<b>←</b>	•	-	4
Lane Group	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	462	190	334	1092	200
v/c Ratio	0.40	0.24	0.33	0.57	0.21
Control Delay	14.5	13.6	1.9	8.0	1.8
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	14.5	13.6	1.9	8.0	1.8
Queue Length 50th (ft)	33	18	0	68	0
Queue Length 95th (ft)	53	37	27	138	21
Internal Link Dist (ft)	517	415		1198	
Turn Bay Length (ft)					
Base Capacity (vph)	2208	1537	1027	1905	967
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.21	0.12	0.33	0.57	0.21
Intersection Summary					

	•	<b>←</b>	•	<b>1</b>	<b>†</b>	ļ	4
Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	161	164	560	579	1203	1860	590
v/c Ratio	0.52	0.53	0.35	0.75	0.50	0.91	0.60
Control Delay	34.9	35.1	0.6	34.8	7.5	34.0	5.2
Queue Delay	0.0	0.0	0.0	0.0	0.8	0.0	0.0
Total Delay	34.9	35.1	0.6	34.8	8.2	34.0	5.2
Queue Length 50th (ft)	77	80	0	139	123	308	0
Queue Length 95th (ft)	126	128	0	179	220	#547	83
Internal Link Dist (ft)		1090			308	844	
Turn Bay Length (ft)	360		360	200			265
Base Capacity (vph)	481	482	1583	925	2426	2035	986
Starvation Cap Reductn	0	0	0	0	815	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.33	0.34	0.35	0.63	0.75	0.91	0.60
Intersection Summary							

<sup># 95</sup>th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Lane Group EBT EBR WBT WBR NBL NBT SBL SBT
Lane Group Flow (vph) 387 102 192 79 482 645 96 1423
v/c Ratio 8.80 0.24 0.80 0.22 1.36 0.54 0.55 1.65
Control Delay 3596.8 9.5 73.4 5.8 216.7 35.7 66.2 328.8
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
Total Delay 3596.8 9.5 73.4 5.8 216.7 35.7 66.2 328.8
Queue Length 50th (ft) ~591 1 150 0 ~513 214 76 ~864
Queue Length 95th (ft) #827 49 236 27 #770 314 134 #1067
Internal Link Dist (ft) 1163 595 462 664
Turn Bay Length (ft) 120 290 105
Base Capacity (vph) 44 434 318 434 355 1196 319 861
Starvation Cap Reductn 0 0 0 0 0 0 0
Spillback Cap Reductn 0 0 0 0 0 0 0
Storage Cap Reductn 0 0 0 0 0 0 0
Reduced v/c Ratio 8.80 0.24 0.60 0.18 1.36 0.54 0.30 1.65

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

## 34: Lenardo Dr & I-405 SB Ramps

	<b>→</b>	•	•	<b>\</b>	1
Lane Group	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	479	311	640	1149	227
v/c Ratio	0.40	0.38	0.55	0.61	0.23
Control Delay	14.4	14.7	2.8	8.6	1.8
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	14.4	14.7	2.8	8.6	1.8
Queue Length 50th (ft)	35	32	0	80	0
Queue Length 95th (ft)	55	57	37	150	22
Internal Link Dist (ft)	517	415		1198	
Turn Bay Length (ft)					
Base Capacity (vph)	2189	1523	1158	1888	972
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.22	0.20	0.55	0.61	0.23
Intersection Summary					

	•	<b>←</b>	•	•	<b>†</b>	ļ	4
Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	98	97	560	865	1335	1025	261
v/c Ratio	0.40	0.39	0.35	0.78	0.49	0.53	0.34
Control Delay	35.5	35.4	0.6	29.9	5.4	22.7	4.7
Queue Delay	0.0	0.0	0.0	0.0	0.5	0.0	0.0
Total Delay	35.5	35.4	0.6	29.9	5.9	22.7	4.7
Queue Length 50th (ft)	48	47	0	196	121	152	0
Queue Length 95th (ft)	90	89	0	248	199	222	54
Internal Link Dist (ft)		1090			308	844	
Turn Bay Length (ft)	360		360	200			265
Base Capacity (vph)	481	483	1583	1108	2740	1929	762
Starvation Cap Reductn	0	0	0	0	865	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.20	0.35	0.78	0.71	0.53	0.34
Intersection Summary							

→ → ←	•	4	<b>†</b>	-	ļ
Lane Group EBT EBR WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph) 326 117 351	157	534	849	200	840
v/c Ratio 6.65 0.27 1.14	0.38	1.58	0.88	0.79	0.95
Control Delay 2595.2 9.0 140.6	19.7	311.9	56.2	75.9	58.4
Queue Delay 0.0 0.0 0.0	0.0	0.0	0.0	0.0	0.0
Total Delay 2595.2 9.0 140.6	19.7	311.9	56.2	75.9	58.4
Queue Length 50th (ft) ~502 0 ~348	39	~647	357	165	302
Queue Length 95th (ft) #696 51 #543	105	#869	#510	248	#432
Internal Link Dist (ft) 1163 595			462		664
Turn Bay Length (ft)	120	290		105	
Base Capacity (vph) 49 429 308	417	337	962	303	889
Starvation Cap Reductn 0 0 0	0	0	0	0	0
Spillback Cap Reductn 0 0 0	0	0	0	0	0
Storage Cap Reductn 0 0 0	0	0	0	0	0
Reduced v/c Ratio 6.65 0.27 1.14	0.38	1.58	0.88	0.66	0.94

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

## 34: Lenardo Dr & I-405 SB Ramps

	<b>→</b>	←	•	<b>&gt;</b>	4
Lane Group	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	468	205	334	1162	200
v/c Ratio	0.40	0.25	0.33	0.61	0.21
Control Delay	14.5	13.7	1.9	8.4	1.8
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	14.5	13.7	1.9	8.4	1.8
Queue Length 50th (ft)	34	20	0	75	0
Queue Length 95th (ft)	54	39	27	152	21
Internal Link Dist (ft)	517	415		1198	
Turn Bay Length (ft)					
Base Capacity (vph)	2205	1535	1026	1902	966
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.21	0.13	0.33	0.61	0.21
Intersection Summary					

	•	<b>←</b>	•	<b>1</b>	<b>†</b>	ļ	4
Lane Group	WBL	WBT	WBR	NBL	NBT	SBT	SBR
Lane Group Flow (vph)	179	182	560	674	1214	1877	590
v/c Ratio	0.56	0.56	0.35	0.80	0.51	0.99	0.62
Control Delay	35.0	35.3	0.6	35.8	8.0	47.8	6.0
Queue Delay	0.0	0.0	0.0	0.0	0.8	0.0	0.0
Total Delay	35.0	35.3	0.6	35.8	8.7	47.8	6.0
Queue Length 50th (ft)	86	88	0	161	130	334	4
Queue Length 95th (ft)	136	138	0	212	231	#567	95
Internal Link Dist (ft)		1090			308	844	
Turn Bay Length (ft)	360		360	200			265
Base Capacity (vph)	481	482	1583	933	2395	1890	950
Starvation Cap Reductn	0	0	0	0	785	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.37	0.38	0.35	0.72	0.75	0.99	0.62
Intersection Summary							

<sup># 95</sup>th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

	-	•	<b>←</b>	•	4	<b>†</b>	-	<b>↓</b>
Lane Group	EBT	EBR	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	393	102	192	79	482	645	96	1423
v/c Ratio	8.93	0.24	0.80	0.22	1.36	0.54	0.55	1.65
Control Delay	3658.6	9.5	73.5	5.8	216.7	35.7	66.2	329.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	3658.6	9.5	73.5	5.8	216.7	35.7	66.2	329.0
Queue Length 50th (ft)	~601	1	150	0	~513	214	76	~864
Queue Length 95th (ft)	#842	49	236	27	#770	314	134	#1067
Internal Link Dist (ft)	1163		595			462		664
Turn Bay Length (ft)				120	290		105	
Base Capacity (vph)	44	434	317	434	355	1196	319	861
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	8.93	0.24	0.61	0.18	1.36	0.54	0.30	1.65

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

## 34: Lenardo Dr & I-405 SB Ramps

	<b>→</b>	<b>←</b>	•	<b>\</b>	4
Lane Group	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	495	321	640	1300	227
v/c Ratio	0.41	0.38	0.55	0.69	0.23
Control Delay	14.5	14.7	2.8	9.8	1.8
Queue Delay	0.0	0.0	0.0	0.0	0.0
Total Delay	14.5	14.7	2.8	9.8	1.8
Queue Length 50th (ft)	36	33	0	99	0
Queue Length 95th (ft)	57	58	37	184	22
Internal Link Dist (ft)	517	415		1198	
Turn Bay Length (ft)					
Base Capacity (vph)	2182	1518	1156	1883	970
Starvation Cap Reductn	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0
Reduced v/c Ratio	0.23	0.21	0.55	0.69	0.23
Intersection Summary					